

2C10 Design for Fire and Robustness

Timisoara, 31.03-11.04.2014

European Erasmus Mundus Master Course

Sustainable Constructions under Natural Hazards and Catastrophic Events



2C10

Examination

- 1. Homework: weight 40 % (dead-line 16 April, 14:00)
- 2. Examination on 17 April, 9:00, room to be announced):
 - a) Written test (10 questions from lectures, without any aids), 60 mins weight 30 %
 - b) Worked examples (choice of 2 examples from 4, all aids available), 90 mins weight 30 %

Notes:

- simple questions covering all 2C10 presentations,
- worked examples similar to homework/ design applications (modifications).



Fire safety

Raul ZAHARIA

Lecture 1: 31/03/2014

European Erasmus Mundus Master Course

Sustainable Constructions under Natural Hazards and Catastrophic Events



Fire safety in buildings

Regulation (EU) No 305/2011 of 9 March 2011

replacing

Construction Products Directive (Council Directive 89/106/EEC)

- Annex I Basic requirements for construction works
- 1. Mechanical resistance and stability
- 2. Safety in case of fire
- 3. Hygiene, health and the environment

EU Regulation No 305/2011

2. Safety in case of fire

The construction works must be designed and built in such a way that in the event of an outbreak of fire:

- a) the load-bearing capacity of the construction can be assumed for a specific period of time
- b) the generation and spread of fire and smoke within the construction works are limited
- c) the spread of fire to neighbouring construction works is limited
- d) occupants can leave the construction works or be rescued by other means
- e) the safety of rescue teams is taken into consideration.

Fire safety in buildings

 The requirements of fire safety are clearly defined, taking into consideration the risk posed by fire and the level of acceptable risk.

- Prescriptive based approach
 - States how a building is to be constructed
- Performance based approach
 - States how a building is to perform under stated criteria

Set of rules for prescriptive approach

For example:

Minimum fire resistance for members

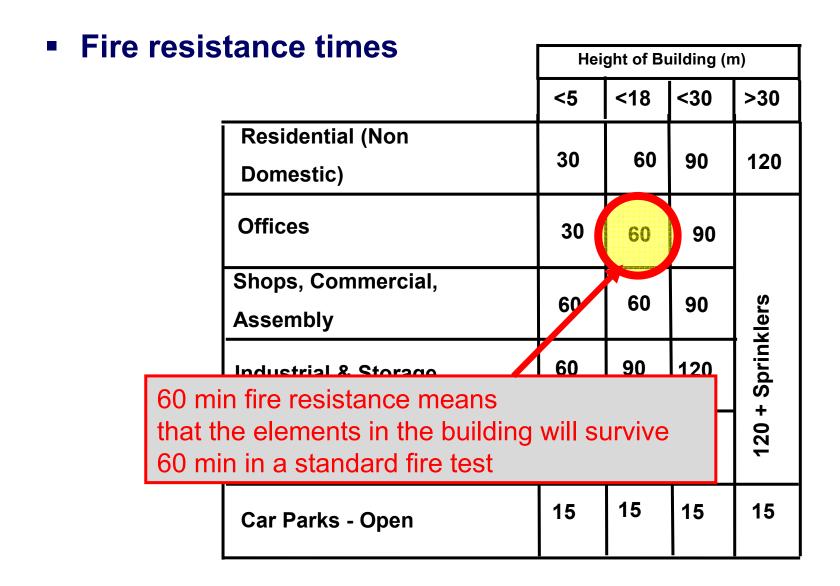
Maximum fire compartment size

Maximum travel distances

Minimum number of exists

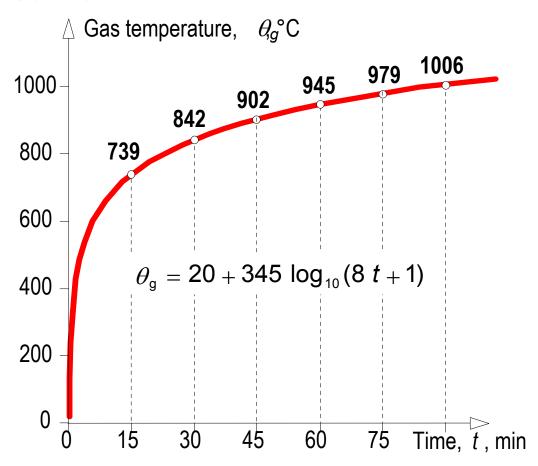
Etc.

Example of prescriptive approach



Standard fire resistance

Heating according to nominal standard (ISO 834) fire curve



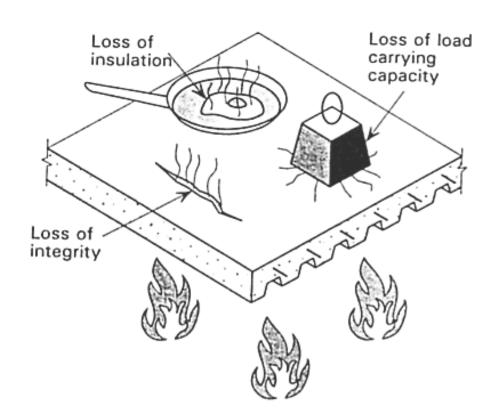
Standard fire resistance

Used for

- Load bearing capacity R
- Insulation
- Integrity

Marked as

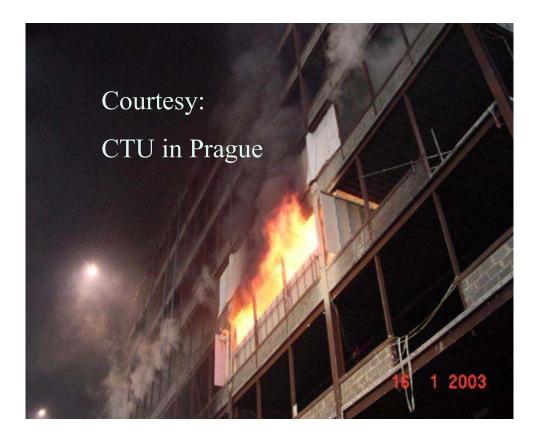
R15; RE30; REI90,...



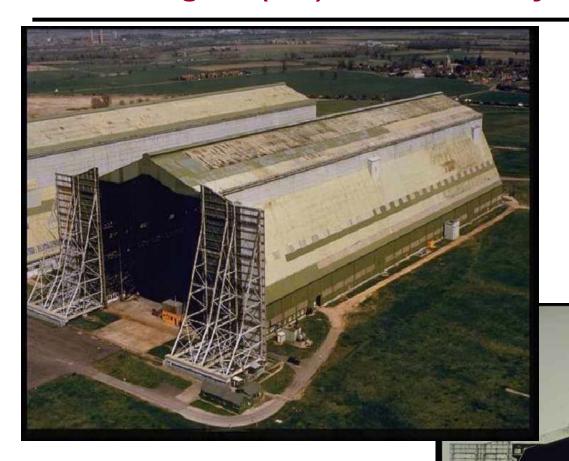
Experimental testing

- Testing material behaviour
- Standard fire tests
- Small scale fire tests
- Large scale fire tests

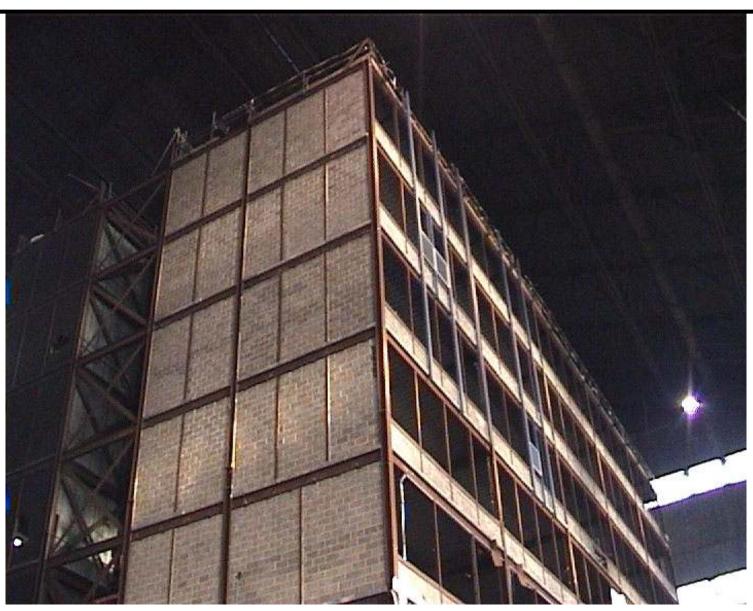
Cardington laboratory test



Cardington (UK) fire laboratory – 48 m x 65 m x 250 m



Steel Composite Structure – 8 Floors



Structure finished 1994, plan area 945 m²



Typical composite structure

beam-column connections - header plates beam-beam connections - fin plates

Fire Compartment







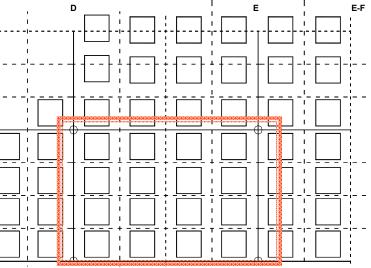
Mechanical Load

- Permanent 100%
- Live around 50% by sand bags









Fire Load

Timber cribs







History of the Standard fire test

Over 100 years of testing

- 1890's early attempts requested by insurance companies
- 1917 First US Standard produced
- **1932** First Edition of BS476 (UK)
- **1933** E119 (US) produced
- **1985** ISO 834
- **2003** EN 1991-1-2



Fire test of intumescent coating

Prescriptive approach - Advantages

- Limited design effort
- Experience has shown that it works
- Easily understood by all parties
- Based on nominal standard fire tests

Prescriptive approach - Disavantages

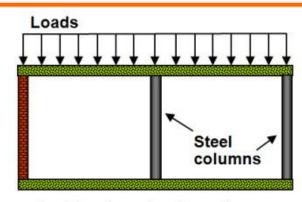
- Actual structural behaviour <u>ignored</u>
- Effect of real fires <u>ignored</u>
- Levels of safety and robustness <u>unknown</u>
- Optimum solution in terms of
 - life safety <u>unknown</u>
 - economical impact <u>unknown</u>
 - environmental damage <u>unknown</u>



Chain of events during a fire



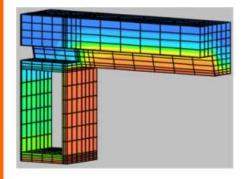




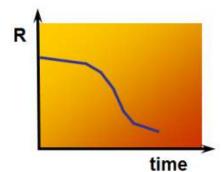
1: Ignition

2: Thermal action

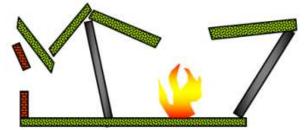
3: Mechanical actions



Thermal response



Mechanical response



6: Possible collapse



Calculation methodologies - Eurocodes

Eurocodes for fire design

Eurocode	At room temperature	In case of fire
0: Basis of design	EN 1990	-
1 : Actions	EN 1991-1-1	EN 1991-1-2
2 : Concrete structures	EN 1992-1-1	EN 1992-1-2
3 : Steel structures	EN 1993-1-1	EN 1993-1-2
4 : Composite steel-concrete structures	EN 1994-1-1	EN 1994-1-2
5 : Timber structures	EN 1995-1-1	EN 1995-1-2
6: Masonry structures	EN 1996-1-1	EN 1996-1-2
7 : Geotechnical design	EN 1997	-
8 : Earthquake resistance	EN 1998	-
9 : Aluminium alloy structures	EN 1999-1-1	EN 1999-1-2

Eurocodes for fire design

- EN 1991-1-2:2002 Actions on structures exposed to fire
- Concept of fire design
- Fire models
- Mechanical loading during fire
- EN 199x-1-2:2005 Structural fire design
- Thermal response
- Material behaviour
- Structural response

Eurocode 2: Design of concrete structures

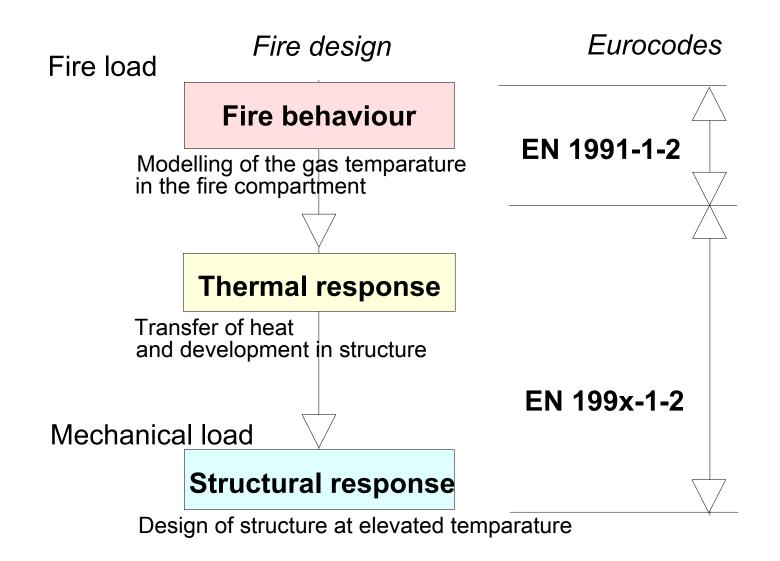
Eurocode 3: Design of steel structures

Eurocode 4: Design of composite steel and concrete structures

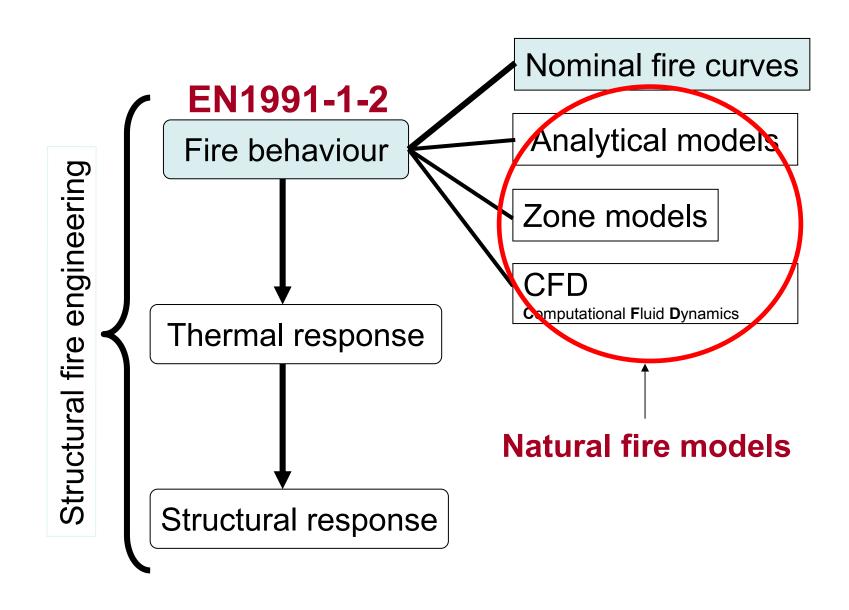
Eurocode 5: Design of timber structures

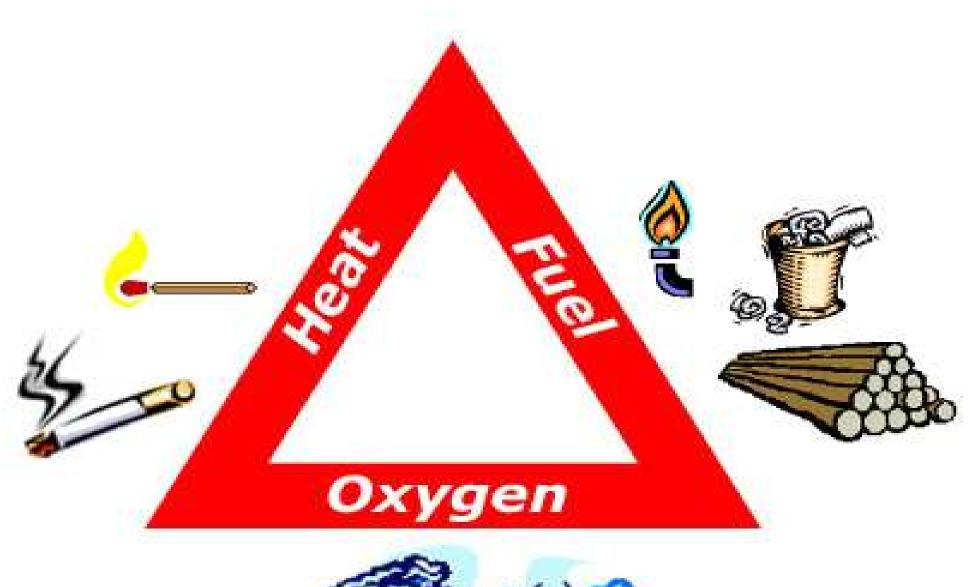
Eurocode 6: Design of masonry structures

Eurocode 9: Design of aluminium structures



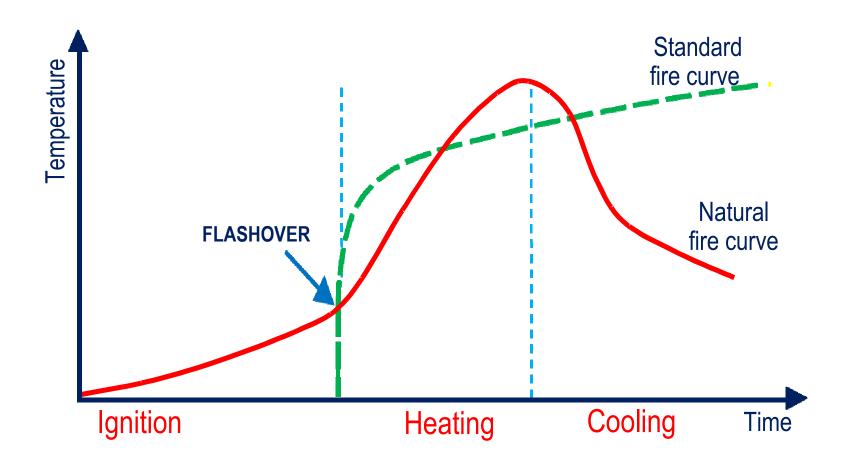
Fire behaviour

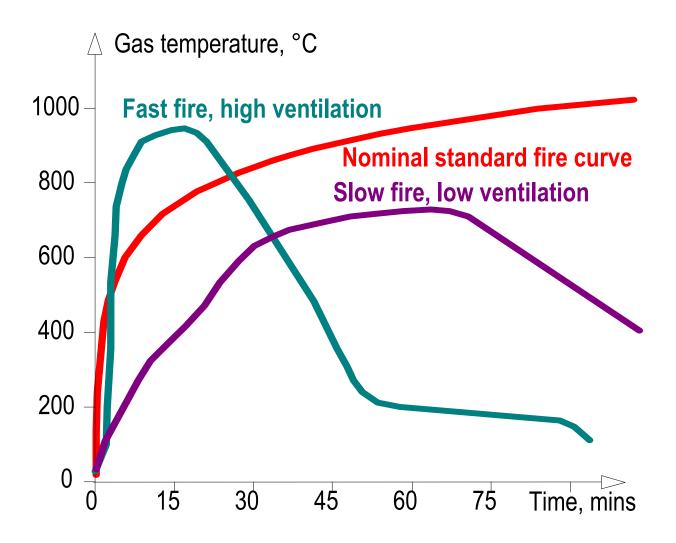






Three phases of natural fire



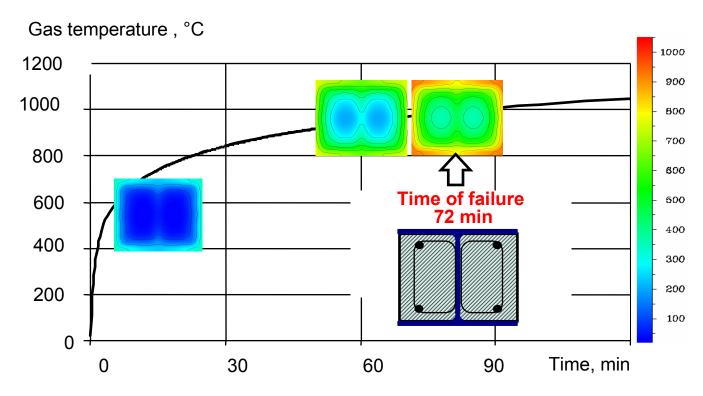


Importance of the fire model considered

For example

Eurocode 4: Composite steel and concrete structures

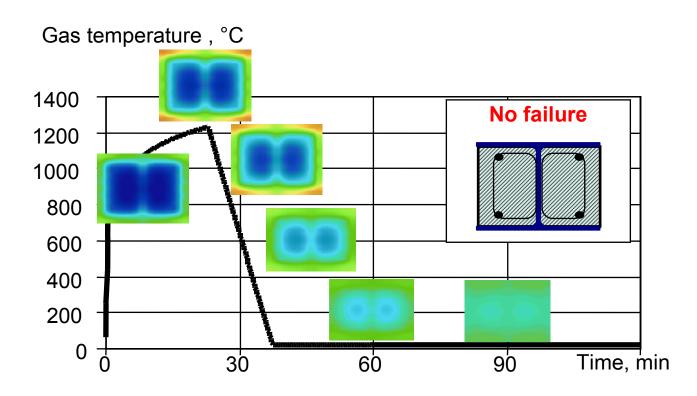
Calculation - Nominal standard fire curve



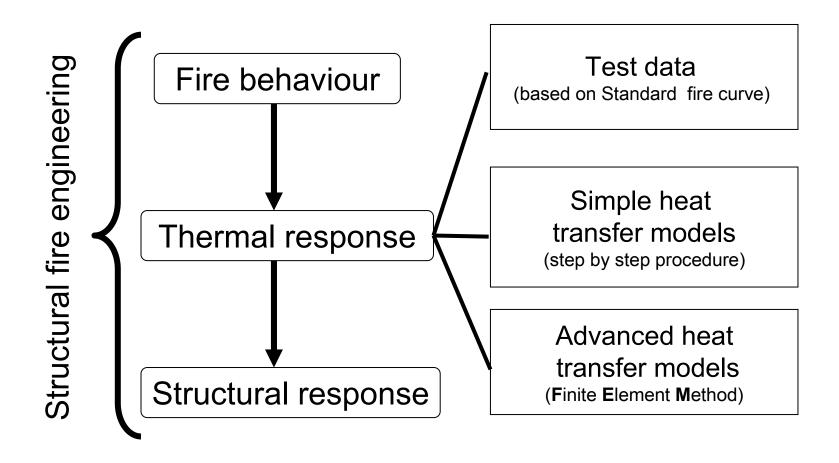
Example

Eurocode 4: Composite steel and concrete structures

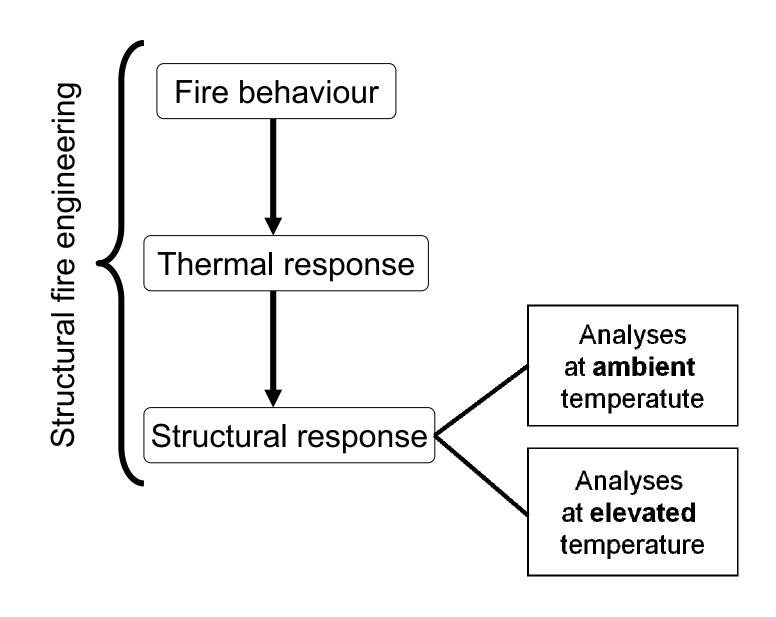
Calculation - Natural fire model: Parametric fire curve



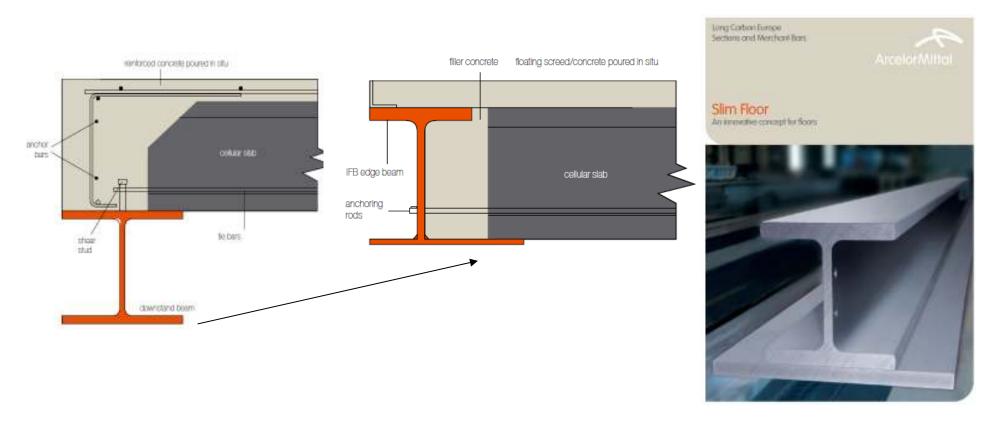
Thermal response



Structural response



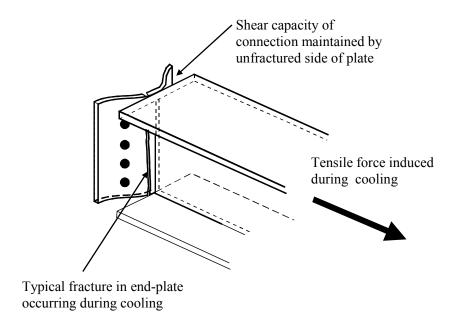
Real structures



- Today buildings are complex
 - New structural solutions
 - New materials
 - Mixed building technology

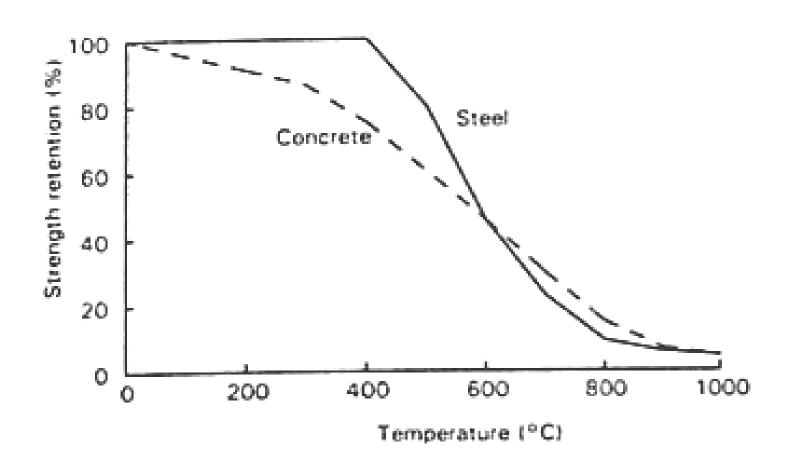
Overall behaviour



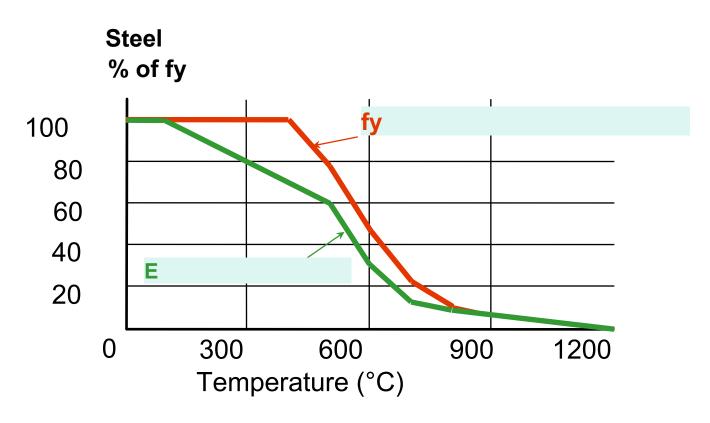


- Cooling phase of fire
 - Connection behaviour
 - The structure may collapse

Material behaviour at elevated temperatures

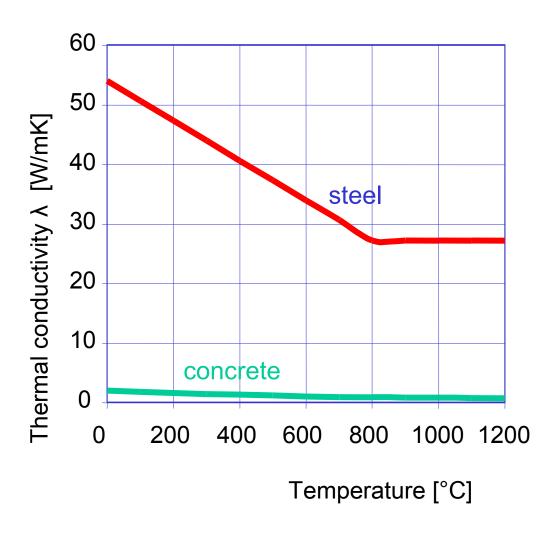


Material behaviour at elevated temperatures



- Young modulus at 600°C reduced aprox. 70%
- Yield limit at 600°C reduced aprox. 50%

Thermal conductivity



Distribution of the temperature on the cross-section

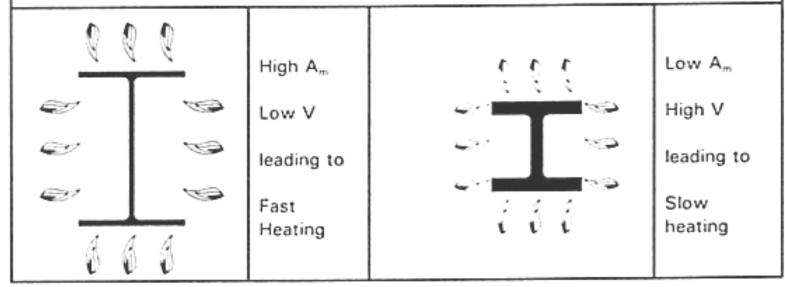
Steel

Section Factor = A_m/V where:

A is the exposed surf

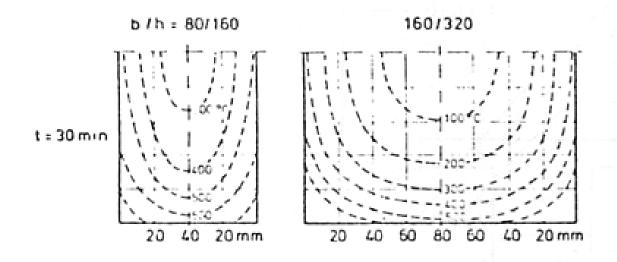
Am is the exposed surface area of the member per unit length;

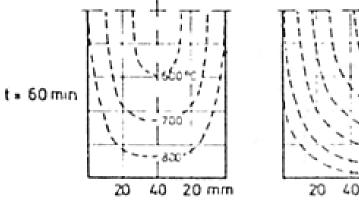
V is the volume of the member per unit length.

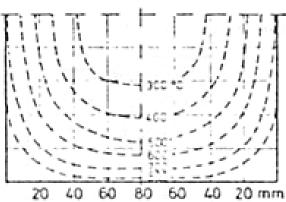


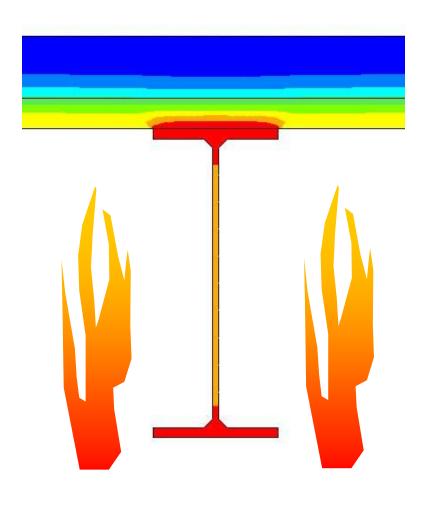
Distribution of the temperature on the cross-section

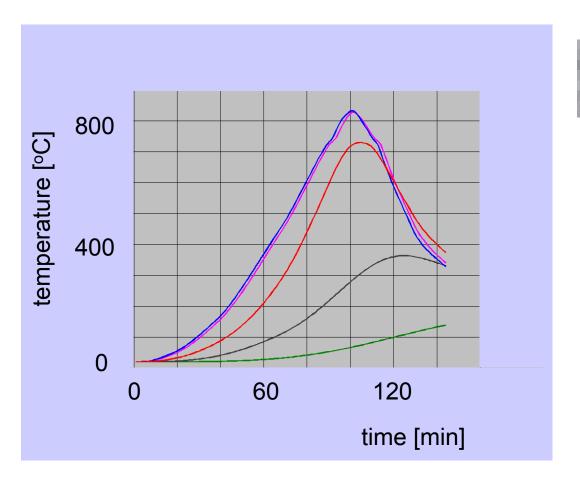
Concrete

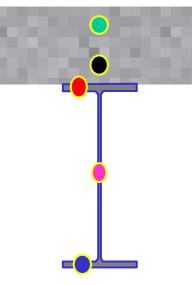












Methods to evaluate the mechanical response of structures in fire

■ Tabulated data

- > Available for concrete and composite
- ☐ Simple calculation models
 - > Critical temperature (only for steel)
 - Available for steel, concrete and composite

<u>Usual</u> <u>applications</u>

Advanced calculation models

- > All types of structures/ materials/ elements
- Numerical models using dedicated computer programs

Advanced design



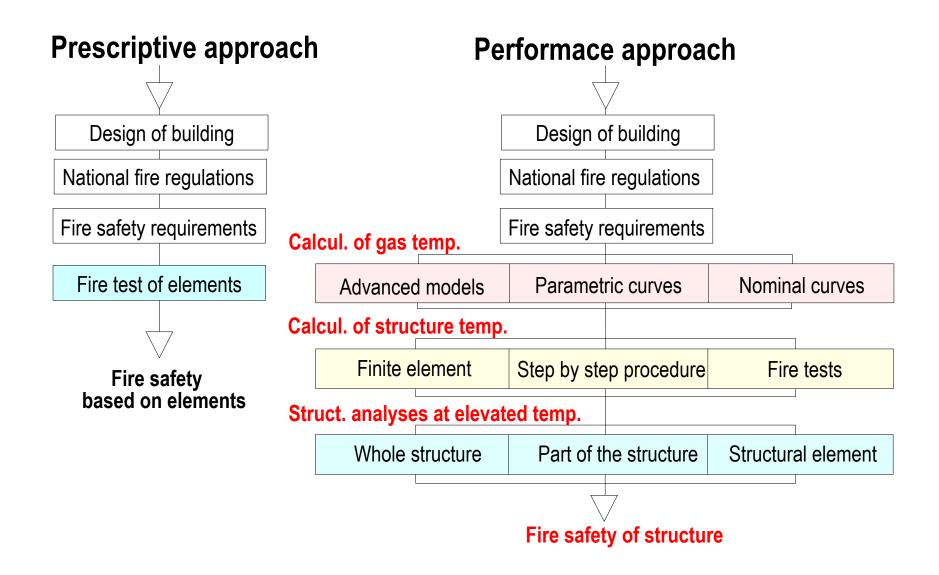
Performance based approach - Avantages

- Actual behaviour and robustness of the building
- Optimum design taking into account
 - Life safety
 - Financial impact
 - Environmental issues
- Part of an assessment of multiple risks
 - Earthquake followed by a fire
 - Explosion followed by a fire

Performance based approach - Disavantages

- More design effort
- Requires multi-discipline skills
- Design can be complicated
- Change of building use may make the fire design invalid

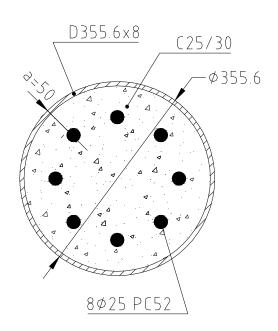
Conclusion



Case study - Application of Eurocodes

Office building for LINDAB-ROMANIA in Bucharest

Field of activity: systems of steel industrial buildings



OWNER DEMAND:

Visible steel - circular columns

FIRE RESISTANCE DEMAND FOR THE COLUMNS: R120



FIRE RESISTANCE OF COMPOSITE COLUMNS

EN 1994-1-2 EUROCODE 4: Design of composite steel and concrete structures

Part 1.2: General rules - Structural fire

design

Tabulated data method

Simple calculation models

Advanced calculation models

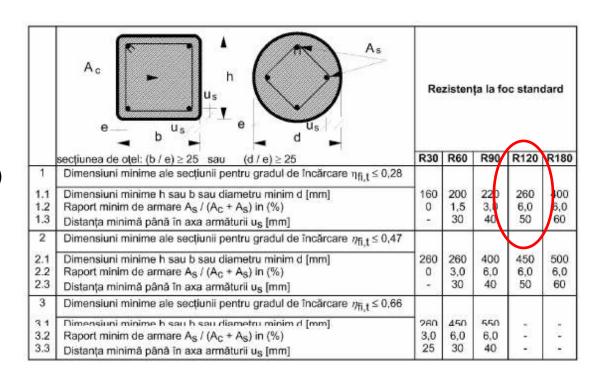
TABULATED DATA METHOD

Minimum characteristics for R120 $(\eta_{fi,t} = 0.3)$:

$$-d_{min} = 260mm < d=355.5 mm$$

$$-\underline{p_{min}} = 6\% > p=4.4\%$$

(load level $\eta_{fi,t} = 0.148$)



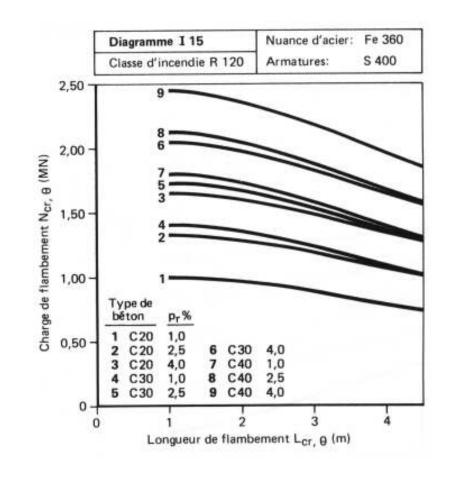
SIMPLE CALCULATION MODEL

(using CIDECT Design Charts for fire resistance of concrete filled hollow section columns)

Fire resistance class R120

- CHS 355.6x5.6
- buckling length $L_f = 3.40m$
- reinforcement p=4%

$$N_{fi,\theta, Rd} \cong 150.000 daN$$
 $> N_{equ} = 61.428 daN$



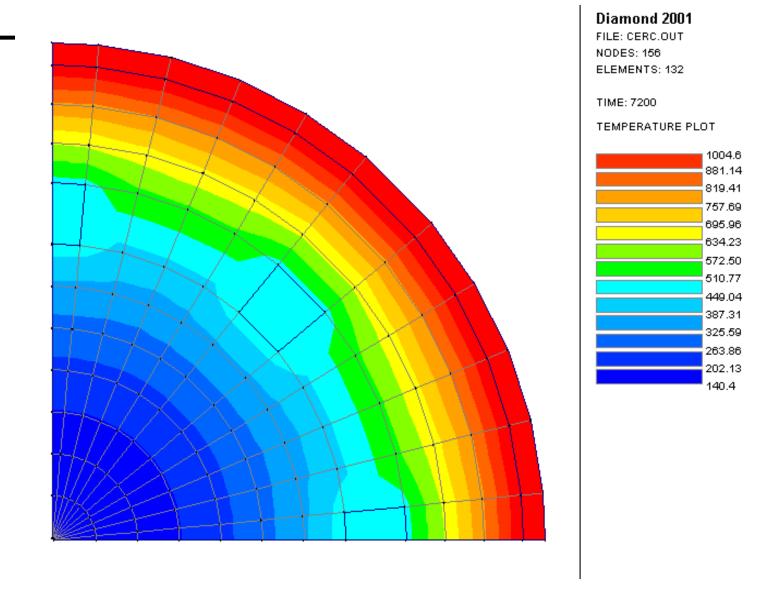
ADVANCED METHOD

SAFIR Program – developed by Prof. Jean - Marc FRANSSEN Liège University, Belgium

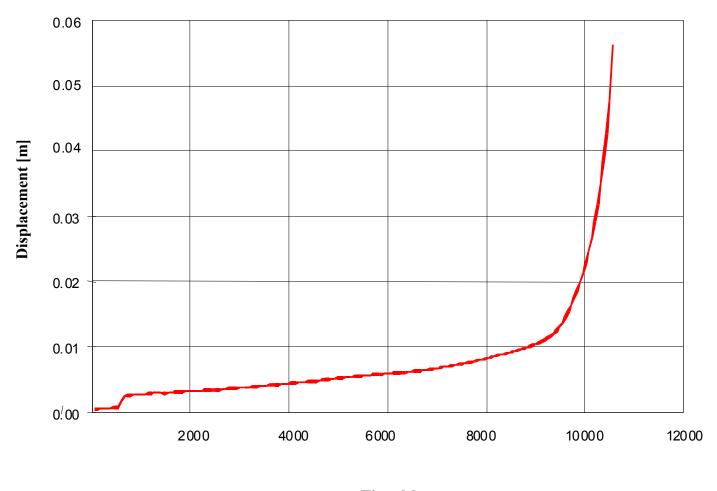
Steps for the analysis of a structure exposed to fire:

First step - predicting the temperature distribution inside the structural members, referred to as 'thermal analysis'

Second step - determining the response of the structure due to static and thermal loading, referred to as 'structural analysis'



Temperature distribution on cross-section after 2h of ISO fire



Time [s]

Time - Displacement characteristic Collapse after 2h56'

Major fire disasters





Great Chicago Fire, 8-10 October 1871



Great Chicago Fire, 8-10 October 1871



World Trade Center 11.09.2001



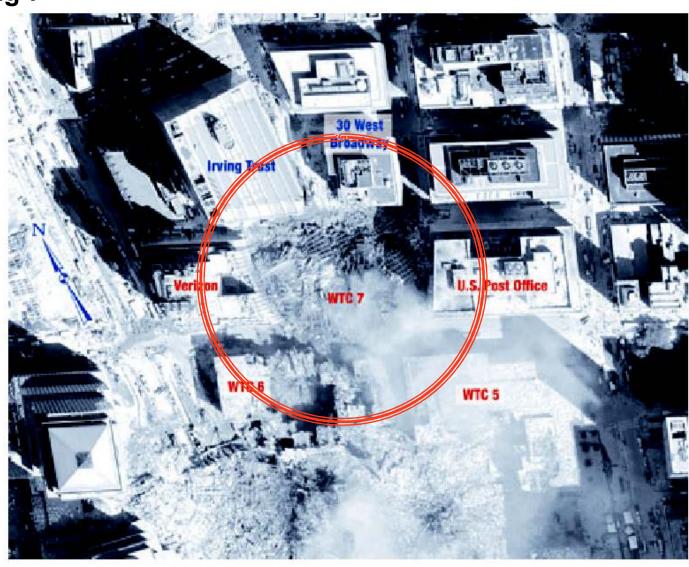




• 17 buildings were damaged



Progresive collapse WTC building 7

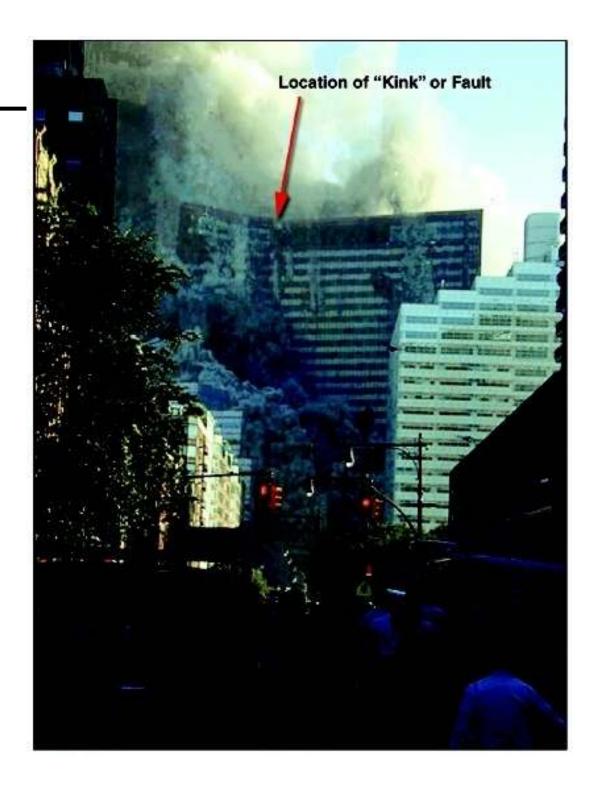


WC7 - large fire





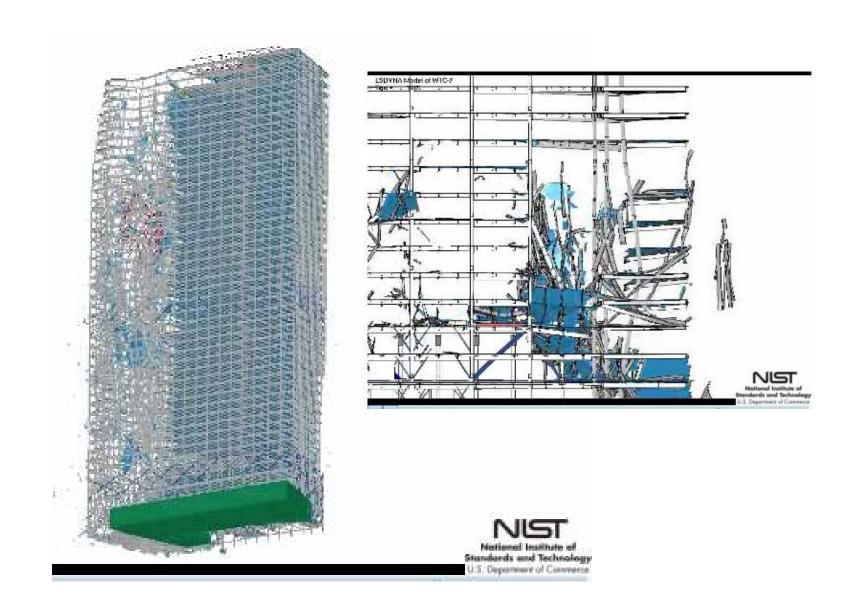
Initial cracking of building



WTC7 progresive colapse



WTC7 – modelling by NIST-US (2008)



WINDSOR TOWER MADRID 13.2.2005



**NO SPRINKLERS FORESEEN IN THIS 100 m HIGH BUILDING



Fire after earthquake

- Fire following earthquake can cause substantially loss of life and property, added to the destruction caused by the earthquake, and represents an important threat in seismic regions
- The loss resulting from fires developing after the earthquake may be comparable to those resulting from the earthquake itself



When an earthquake occurs:

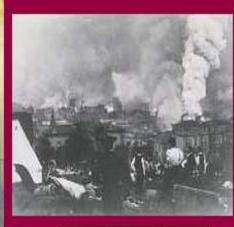
- it may cause structural and fire protection damage, and thus the building is more vulnerable to a fire
- the loss of water supply or the low water pressure, combined with multiple independent fires, traffic congestion and the limited resources which are not able to respond promptly to all fires, allow to the fire to spread
- the experience of the impact of fire following earthquake indicates that at wind speeds above 9m/s, the fire spread and associated loss increase dramatically
- the increased time needed for firemen to reach the building in fire, associated to the possible lack of the active fire measures and to the increased vulnerability of the building may affect their security

San Francisco, California, 1906

- 7.8 Richter magnitude earthquake
- the "Great fire" scorched 508 city blocks, or 4.7 square miles of San Francisco
- "Firestorms fed by fierce winds raged for three days in San Francisco and caused the majority of the damage.."
- "... the earthquake and fire combined to cause the nations' greatest urban tragedy..."

San Francisco, California, 1906

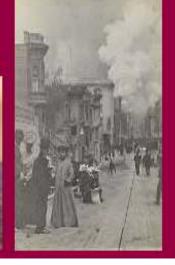






(The Bancroft Library)





(Courtesy of The San Francisco Museum of Modern Art)

The estimated fire-induced loss is 10 times larger than the one directly induced by the ground motion

Bucharest, Romania, 1977

- 7.2 Richter magnitude earthquake the strongest in an European capital
- Charles Richter about Vrancea region (Romania): "Nowhere in the world doesn't exist such a exposed population to the earthquake from the same source".
- 29 isolated fires, on partially affected by the earthquake or collapsed buildings

Kobe, Japan, 1995

- 7.2 Richter magnitude earthquake
- "When the earthquake hit, fire broken out .. Over 300 fires quickly started, especially among the remains of the wooden buildings"
- "At least a dozen major fires that burned for up to two whole days... before they were brought under control."
- "Research conducted at Kobe University suggests that 500 deaths were due to fires, and that almost 7000 buildings were destroyed by fire alone."

Kobe, Japan, 1995











Almost 7000 buildings were destroyed by fire alone





Learning from disasters

- Proper prescriptive rules are very important
- New materials and new structural solutions need performance based design
- In fire design should be integrated
 - Life safety
 - Financial impact
 - Environmental issues
- Multiple risks
 - Earthquake followed by a fire
 - Explosion followed by a fire

Active and pasive fire protection measures

Active:

- Sprinklers or other active systems like rapid smoke evacuation reduce the risk and severity of a fire
- Detection and alarm systems achieve more rapid evacuation
- etc

Passive:

- Proper compartimentation: an atrium or other large internal enclosure positively influences the effective compartmentation in fire
- Proper means of escape ensure the evacuation in safe conditions
- Adequate fire resistance of elements- fire protection
- etc

Forms of passive fire protection

There are five forms of passive fire protection:

- Spray protection applied around the profile
- Board protection applied as a 'box'
- Intumescent coating applied around the profile
- Concrete encasing forms a rectangular encasement around the member
- Composite member for example in-filling of tubular columns



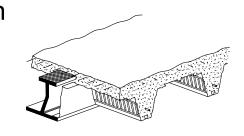




Composite sections

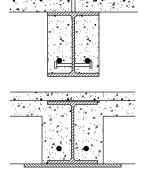
Filled sections till fire resistance 60-90 min Additional reinforcement 120 min

Slab



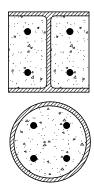
Beams

Patially encased Slim floor



Columns

Partially encassed Concrete filled



Recapitulation

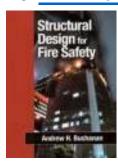
- Which are the major topics related to fire in EU Construction products directive?
- Which are the differencies between the prescriptive approach and the performance based approach for the fire safety of buildings?
- Which parts of Eurocodes are focussed for fire design and which Eurocodes include fire design?
- In which Eurocode is described the fire loading? How many types of fire models exist?
- Which are the levels of verification methods in the Eurocodes for fire design?
- Which is the difference between the passive and active fire protection measures? Give some examples

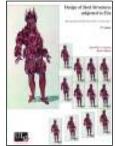
Further reading

 Further readings on the relevant documents from websites of www.access-steel.com and www.difisek.eu.

Books

Buchanan A. H.,
 Structural Design for Fire Safety,
 Wiley, 2001, ISBN 0471889938.





- Franssen J.M., Zaharia R., Design of steel structures subjected to fire – Background and design guide to EN1993-1-2, Les Editions de l'Universite de Liege, 2005, ISBN 9782874560279
- D. Moore, T. Lennon, C. Bailey, Y. Wang, Designers' Guide to EN 1991-1-2, EN 1993-1-2 and EN 1994-1-2: Handbook for the Fire Design of Steel, Thomas Telford, 2007, ISBN 0727731572.
- Franssen J.M., Kodur V., Zaharia R., Designing steel structures for fire safety, Taylor & Francis – Balkema, 2009, ISBN 978041554828
- Fransen J.M., Vila Real P.,
 Fire design of steel structures,
 ECCS 2011, ISBN978-92-9147-099-0.
- Wang Y., Burgess I., Wald F., Gillie M.,
 Performance Based Fire Engineering of Structures
 CRC Press 2012, ISBN: 978-0-415-55733-7.







Selection of fire engineering strategy may be find at Acces-Steel

- •Client guide on the key issues for structural fire resistance
- Scheme development
 - Fire safety strategy for multi-storey buildings for commercial and residential use
 - Selection of appropriate fire engineering strategy for multistorey commercial and apartment buildings
 - Checklist for fire design of multi-storey office buildings
 - Checklist for fire design of multi-storey apartments
 - Fundamentals of structural fire design
 - Ensuring fire safety





Thank you for your attention

Raul ZAHARIA raul.zaharia@upt.ro