

Universitatea Politehnica Timișoara

Facultatea de Construcții

Departamentul de Căi de Comunicație Terestre, Fundații și Cadastru



FOUNDATIONS

- CURS 8 -

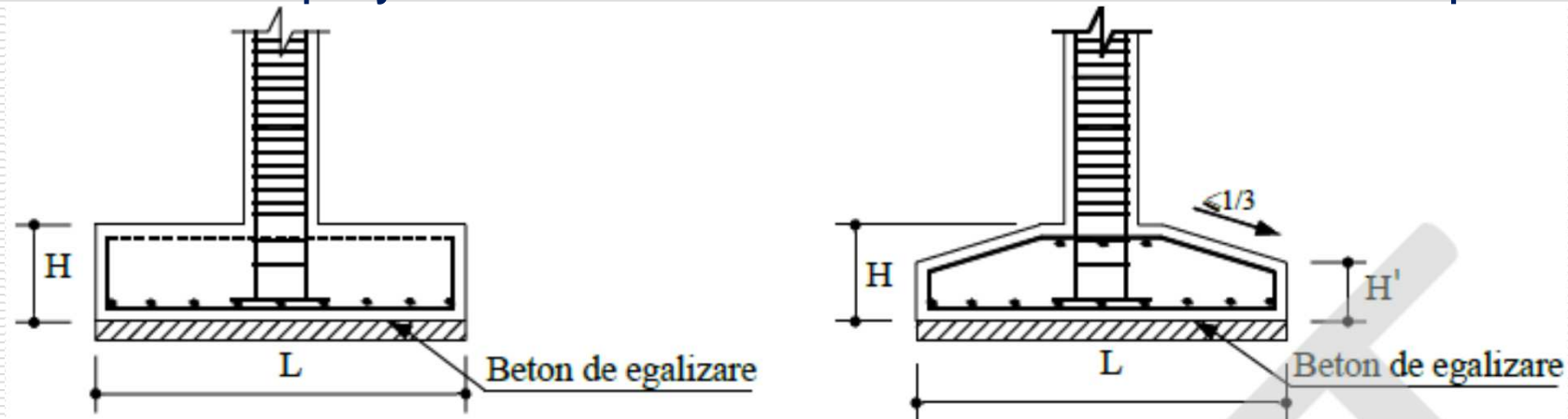
Design of shallow foundations

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CHAPTER VI – SHALLOW FOUNDATIONS

§ 6.5 Design of reinforced concrete block foundations

- **Reinforced concrete block foundations** (also known as **elastic foundations**) are realized as reinforced blocks under cast-concrete columns.
- This type of foundation is generally used when the columns have high bending moments or when the water level is high.
- The reinforced concrete block foundations could be realized as prismatic or splayed blocks in function of their dimensions in plan.

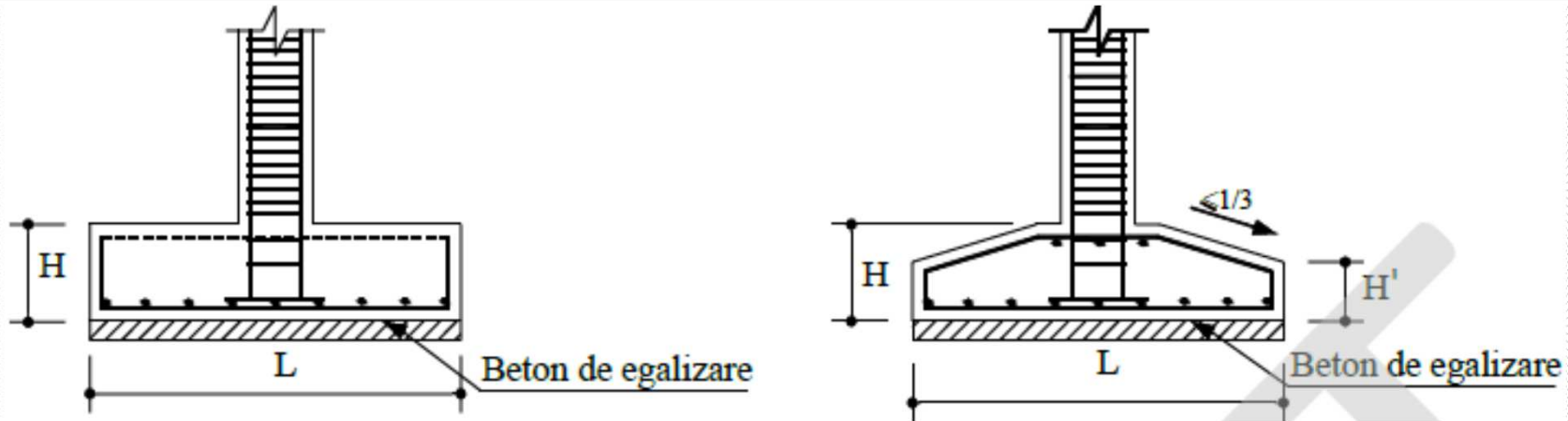


Obs: In order to obtain a horizontal footing level, a 5 to 10 cm equalizing concrete layer is cast (C8/10).

CHAPTER VI – SHALLOW FOUNDATIONS

§ 6.5 Design of reinforced concrete block foundations

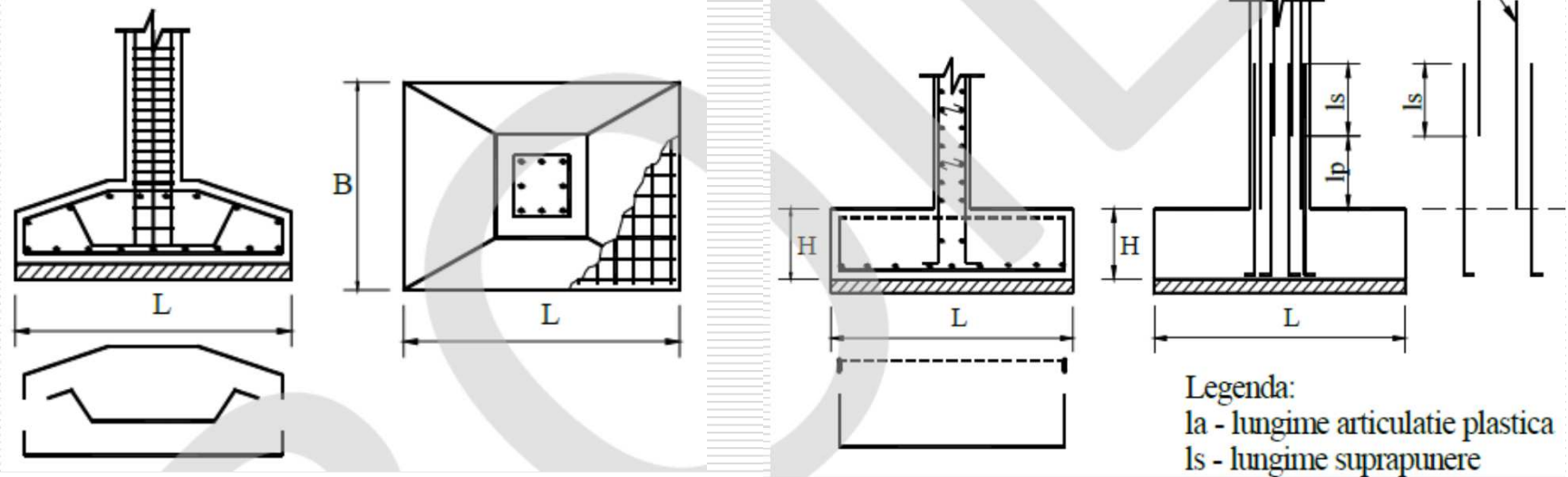
□ Minimum geometrical requirements



- Minimum height: $H_{min} = 300mm$
- In case of splayed concrete blocks: $H'_{min} = 250mm$
- In case of splayed concrete blocks: slope of inclined faces $< 1/3$
- Minimum concrete class: min C12/15

§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

□ Reinforcement in block foundations:



□ Sole reinforcement:

Results from the design to bending moment of the foundation.

Minimum requirements: $\rho_{min}=0.1\%$, $\Phi_{min}=10mm$, $d_{min}=100mm$,
 $d_{max}=250mm$, anchoring hooks of minimum height (d).

§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

□ Reinforcement in block foundations:

□ **Superior reinforcement:**

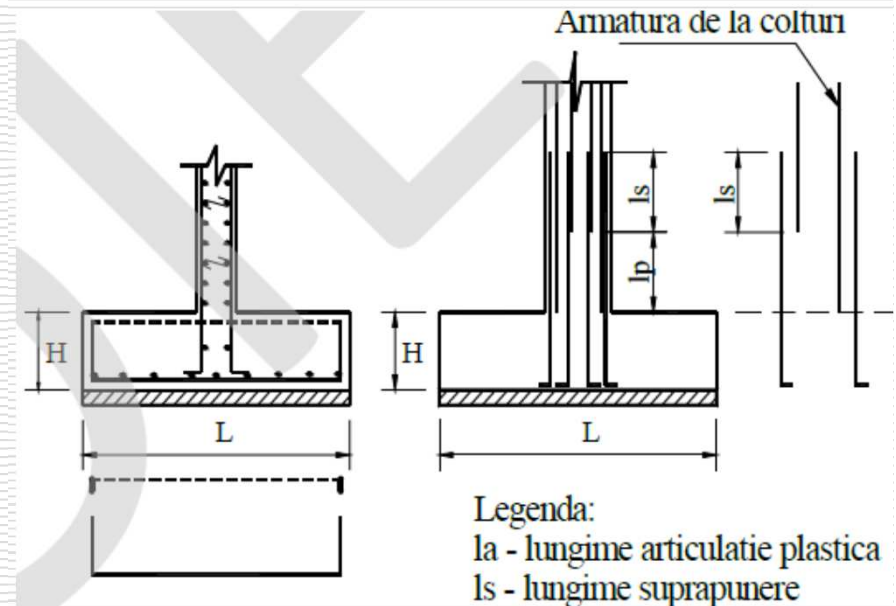
At least three bars neat the column or grid distributed on foundation:

- constructive for foundations working in compression;
- resulting from bending check of the foundation block, considered as a cantilever and loaded by soil pressure

□ **Column reinforcement:**

Identical to the column reinforcements. Their design results from the column design.

- the reinforcing bars are extended beyond the length of column plastic hinge;
- over this zone, the bars are driven over a length $l_{bd}=30\Phi$



Minimum requirements: $\Phi_{min}=10mm$,
 $d_{min}=100mm$, $d_{max}=250mm$,

§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

□ Reinforcement in block foundations:

□ **Transversal reinforcement:**

Needed to overtake the shear foundation forces or results from punching design.

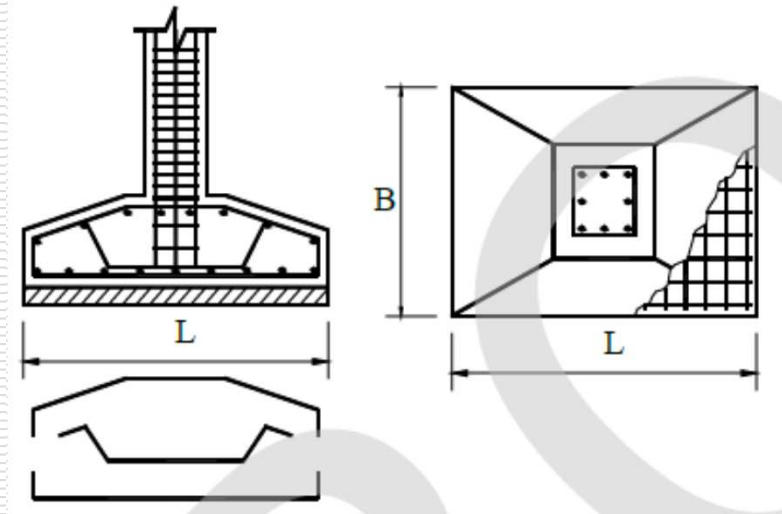
Is realized as inclined bars under the column.

□ **Stirrups:**

Their purpose is to fix the column reinforcement. They are identical to column stirrups.

Minimum requirements: at least three stirrups at maximum distance of 250mm.

Obs: The proper foundation reinforcement does not need stirrups. The foundation reinforcement results as a reinforcement cage.



§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

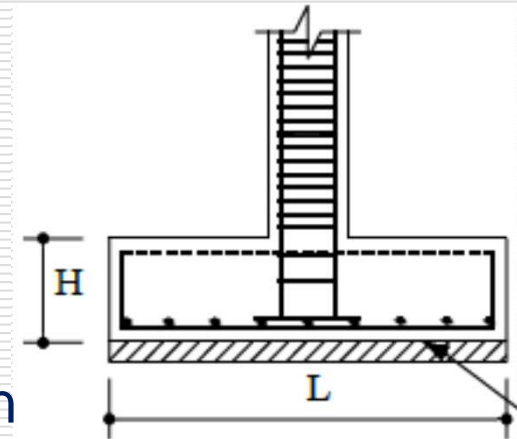
□ The dimensions of the foundation block results from the minimum requirements or from different checks characteristic for such systems.

□ **Condition of rigid foundation:**

If $H/L \geq 0.3$ then we can assume the hypothesis of the linear distribution of the ground pressures.

□ **Foundation check to shear force:**

If the following relationship is fulfilled, the foundation does not need transversal reinforcement (characteristic to resist in shear): $V_{Ed} \leq V_{Rd,c}$



with: V_{Ed} – maximum shear force, resulted from foundation analysis

$V_{Rd,c}$ – concrete shear capacity: $V_{Rd,c} = 0,12k \cdot (100\rho_1 f_{ck})^{1/3} \cdot B \cdot H$

but $V_{Rd,c} \geq v_{min} \cdot B \cdot H = 0,035k^{3/2} \cdot f_{ck}^{1/2} \cdot B \cdot H$

§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

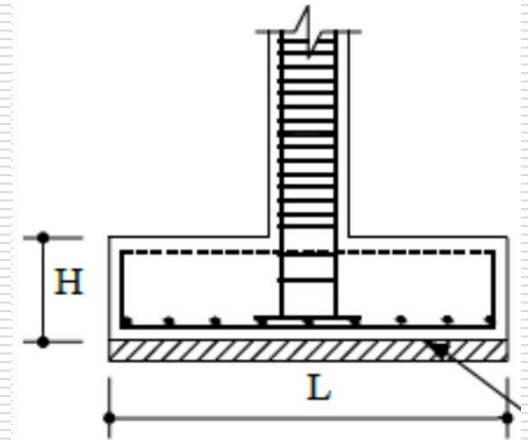
$$V_{Rd,c} = 0,12k \cdot (100\rho_1 f_{ck})^{1/3} \cdot B \cdot H$$

with: $k = 1 + \sqrt{\frac{200}{d}} \leq 2$ $\rho_1 = \frac{A_{sl}}{Bd} \leq 0,02$

B – foundation width

$d = H - a$ is the effective height of the foundation

A_{sl} – area of the longitudinal reinforcement extended on a length $l \geq lbd + d$



Obs: If the above condition is not fulfilled, adequate shear reinforcement should be provided. This results from concrete design of the foundation.

§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

□ Foundation check to punching:

If the following relationship is fulfilled, the foundation does not need transversal reinforcement against punching:

$$v_{Fd} \leq v_{Rd,c} = 0,12k \cdot (100\rho_1 \cdot f_{ck})^{1/3}$$

This check is considered on a perimeter u_1 , located at a distance $2d$ from the column edge.

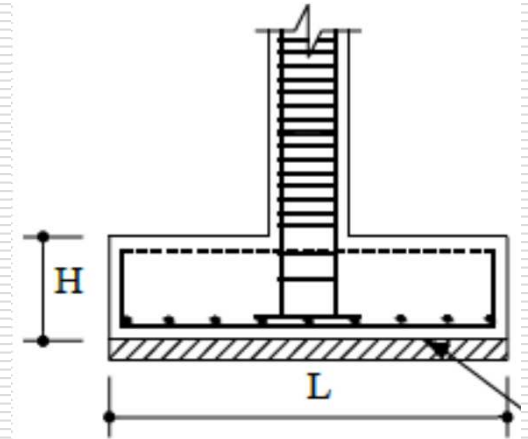
with: $\rho_1 = \sqrt{\rho_x \rho_y}$ ρ_x, ρ_y the reinforcement percent on x and y directions

$$v_{Fd} = \beta \frac{N_{Fd}}{u_i d}$$

β - a coefficient that accounts for the distribution of bending moments: $\beta=1.15$ for central columns and $\beta=1.5$ other situations.

If needed, other punching checks should be performed, on perimeters located at distances smaller than $2d$. In this case $v_{Rd,c}$ will be multiplied with $2d/a$ (a – distance from the column edge).

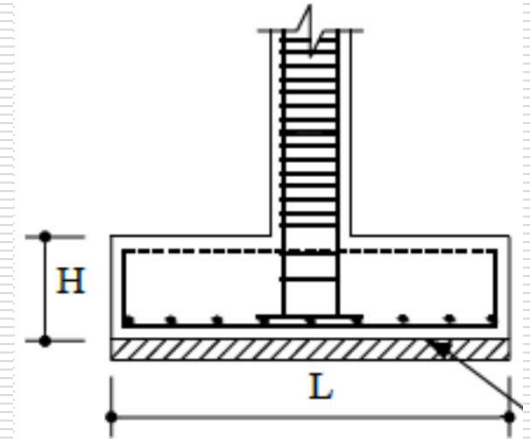
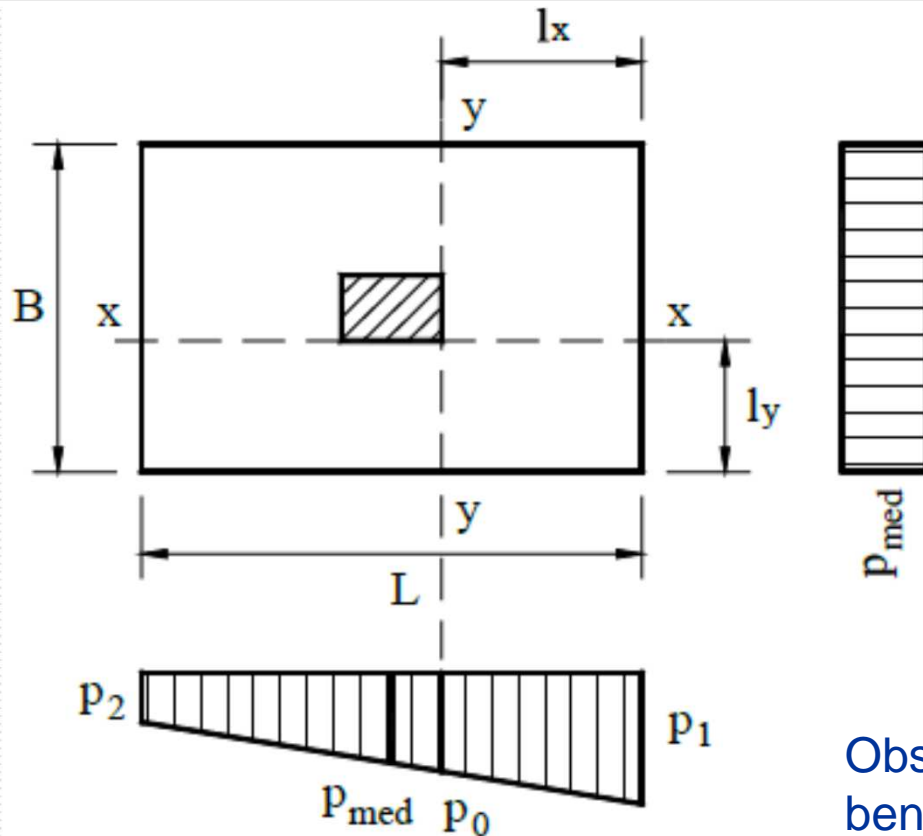
Obs: If the above condition is not fulfilled, adequate punching reinforcement should be provided. This results from concrete design of the foundation.



§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

□ Design of sole reinforcement:

The number and distribution of sole reinforcement bars results from the values of bending moments:



Axial load with eccentric bending

$$M_{Fd,x} = B \cdot \left[p_0 \frac{l_x^2}{2} + (p_1 - p_0) \frac{l_x^2}{3} \right]$$

$$M_{Fd,y} = L \cdot p_{med} \cdot \frac{l_y^2}{2}$$

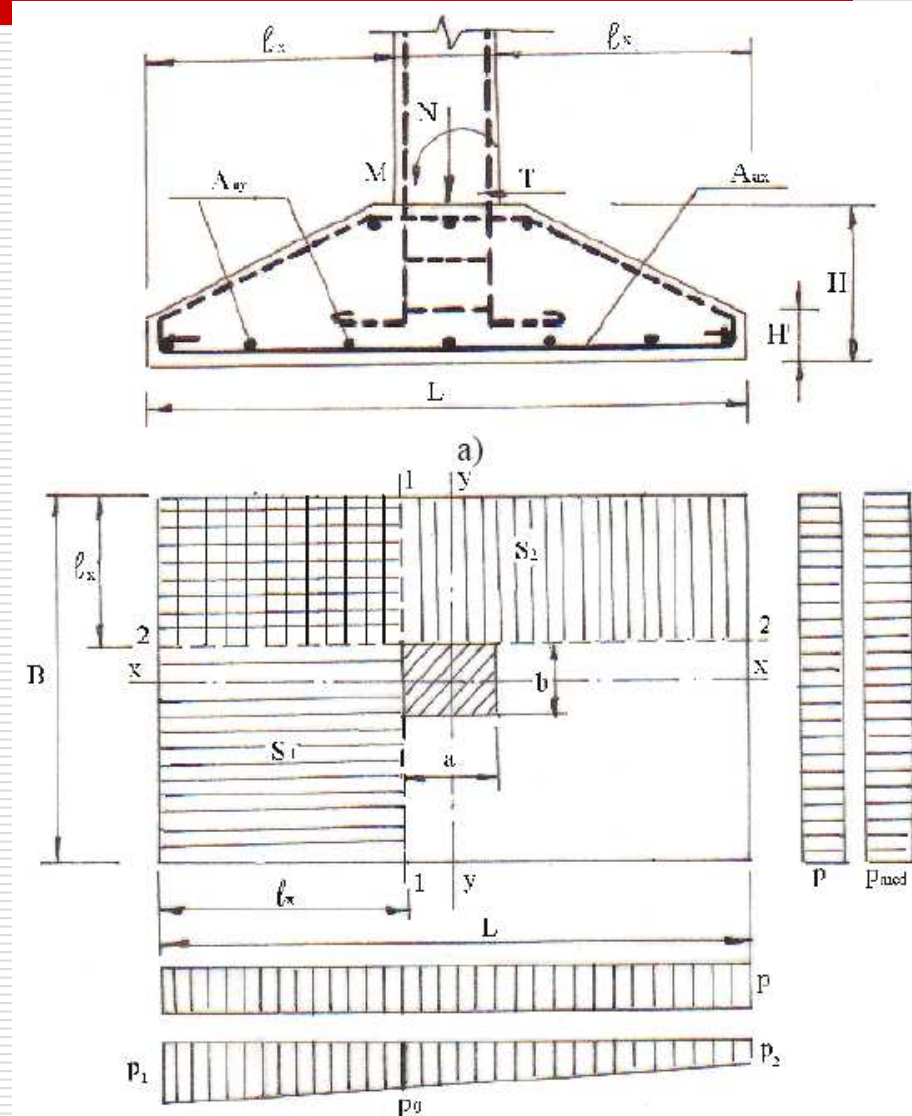
With: $p_{med} = (p_1 + p_2) / 2$

Obs: Different formulae are derived for eccentric bending on two directions.

§ 6.5 Design of reinforced concrete block foundations (NP 112/2014)

- ❑ **Design of sole reinforcement:**
- ❑ In this design, it is considered that the sole is loaded by the geological pressure on a bandwidth l_x (l_y)
- ❑ Maximum bending is obtained in cross-sections 1-1 and 2-2 located at the column edge.
- ❑ The design is conducted following the usual reinforced concrete rules.

Obs: Upper reinforcement is not needed. However, distribution reinforcement is provided against concrete cracking.

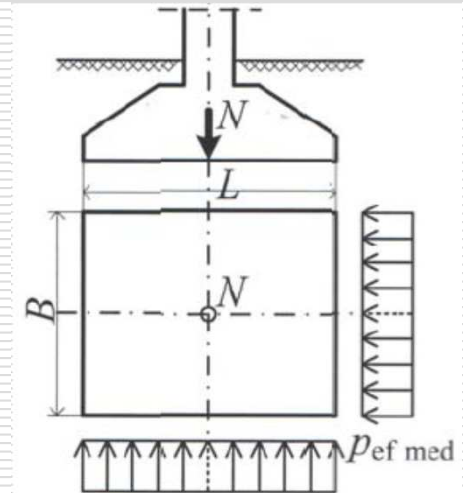


§ 6.5 Design of reinforced concrete block foundations (NP 112/2013)

□ **Check of soil pressure :**

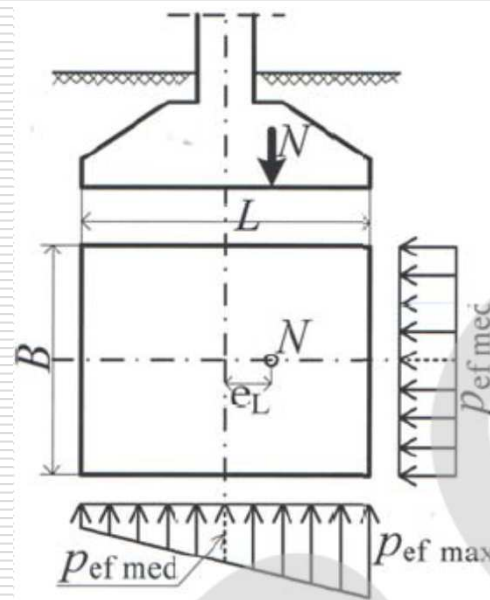
□ Centric loading:

$$p_{ef\ med} = N_F / A \leq p_{conv}$$



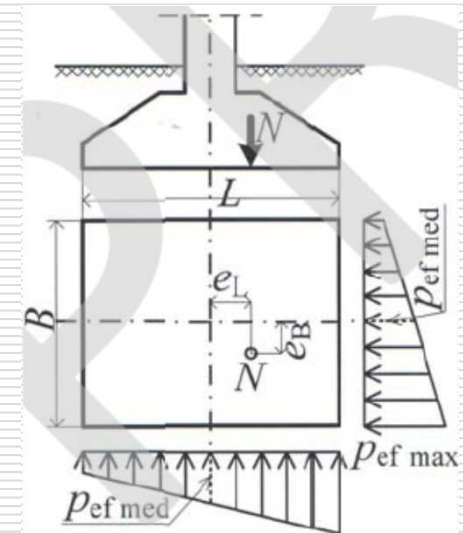
□ Uni-axial eccentric loading:

$$p_{ef\ max} = \omega \frac{N_F}{L \cdot B} \leq 1,2 p_{conv}$$



□ Bi-axial eccentric loading:

$$p_{ef\ max} = \omega \frac{N_F}{L \cdot B} \leq 1,4 p_{conv}$$



Obs: From these formulae is excluded the seismic (special) combination of actions.