

FOUNDATIONS

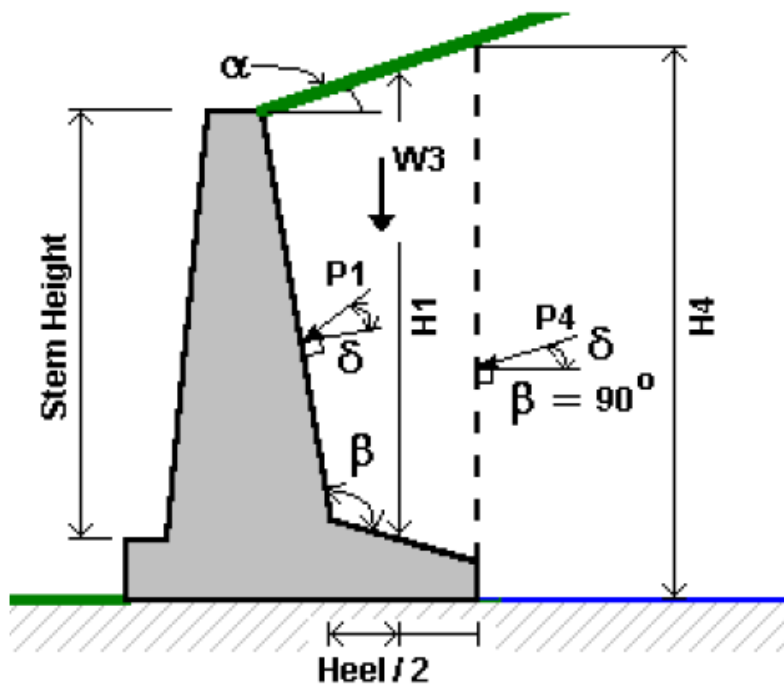
- CURS 6 -

Lateral Earth Pressure and
Retaining Walls

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CHAPTER V – LATERAL EARTH PRESSURE AND RETAINING WALLS

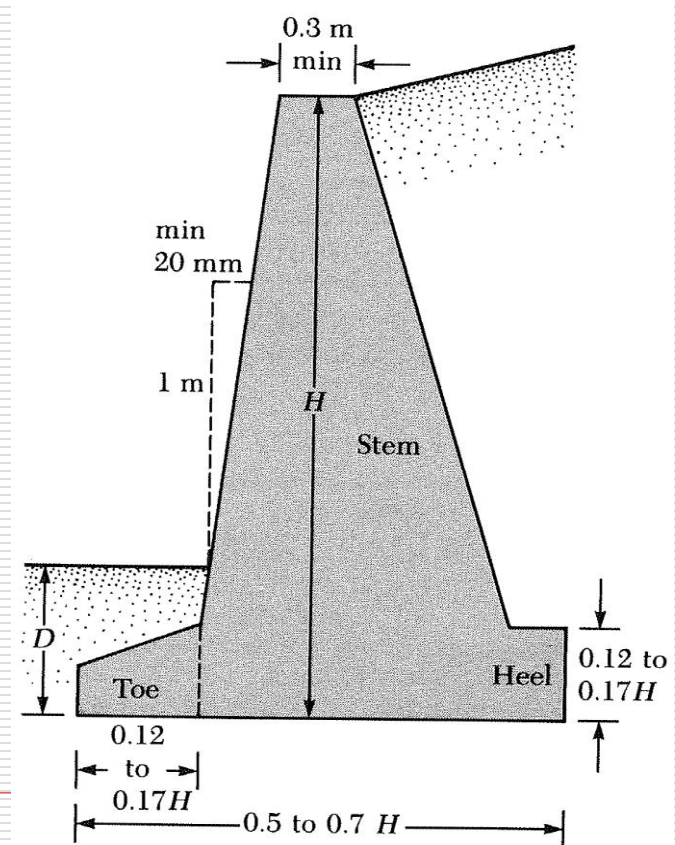
□ A **retaining wall** is a wall that provides lateral support for a vertical or near vertical slope of soil.



CHAPTER V – LATERAL EARTH PRESSURE AND RETAINING WALLS

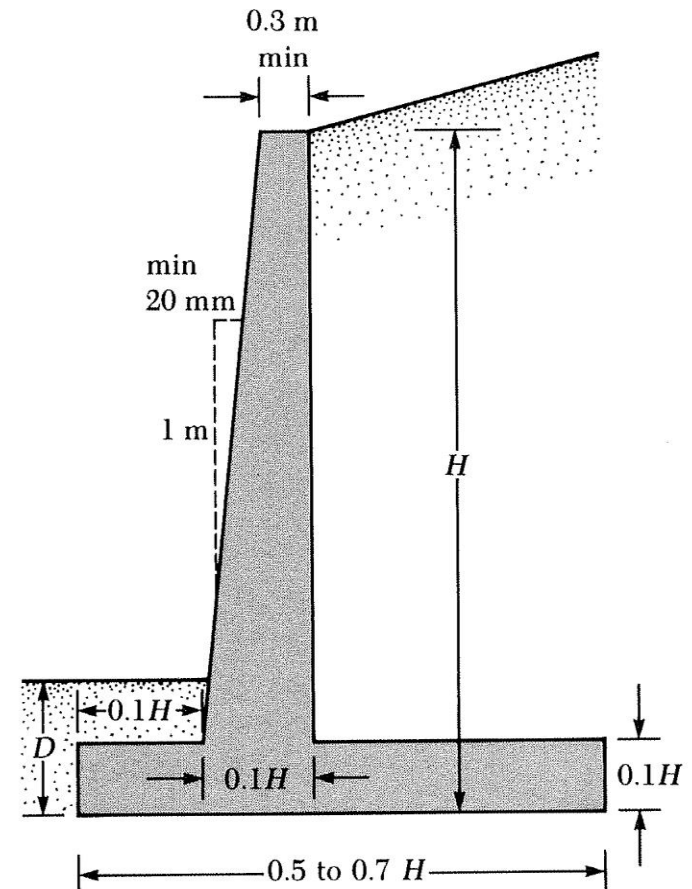
§ 5.3 Proportioning of retaining walls

- ❑ When designing retaining walls, one must assume some of its dimensions (*proportioning*).
- ❑ Proportioning allows the engineer to check trial sections for stability in an iterative process.
- ❑ For gravity RW:
 - ❑ Top: min 0.3m
 - ❑ Sole length: $0.5-0.7H$
 - ❑ Heel height: $0.12-0.17 H$
 - ❑ Toe length: $0.12-0.17 H$
 - ❑ Frost depth (D): in accordance with the local frost conditions (not less than 0.6m)



§ 5.3 Proportioning of retaining walls

- For counterfort and cantilever RW:
 - Top: min 0.3m
 - Sole: 0.5-0.7H
 - Heel height: approx. 0.1H
 - Toe length: approx. 0.1H
 - Frost depth (D): in accordance with the local frost conditions (not less than 0.6m)
 - counterfort slabs: aprox. 0.3m, spaced center to center 0.3-0.7H.



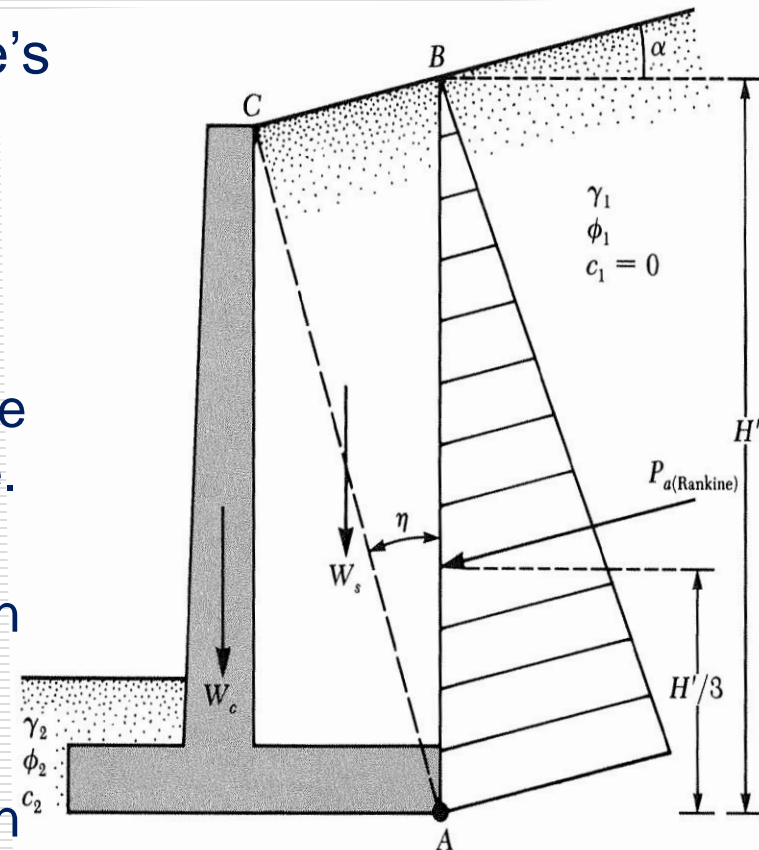
§ 5.4 Design of retaining walls

Application of Lateral Earth Pressure to Design

□ For the design of RW, the general theories of application of lateral earth pressure on RW should be made on the basis of some assumptions:

□ In case of cantilever RW, if the Rankine's earth pressure is to be used, then:

- The active pressure is considered as acting on the vertical line starting from the edge of the heel.
- The Rankine active pressure acts on the line AB, constructed from the heel edge.
- The weight of the wall slabs and the weight of the soil above the heel acts on their centroids.
- The weight of the wall slabs and the weight of the soil above the heel acts on their mid-widths.



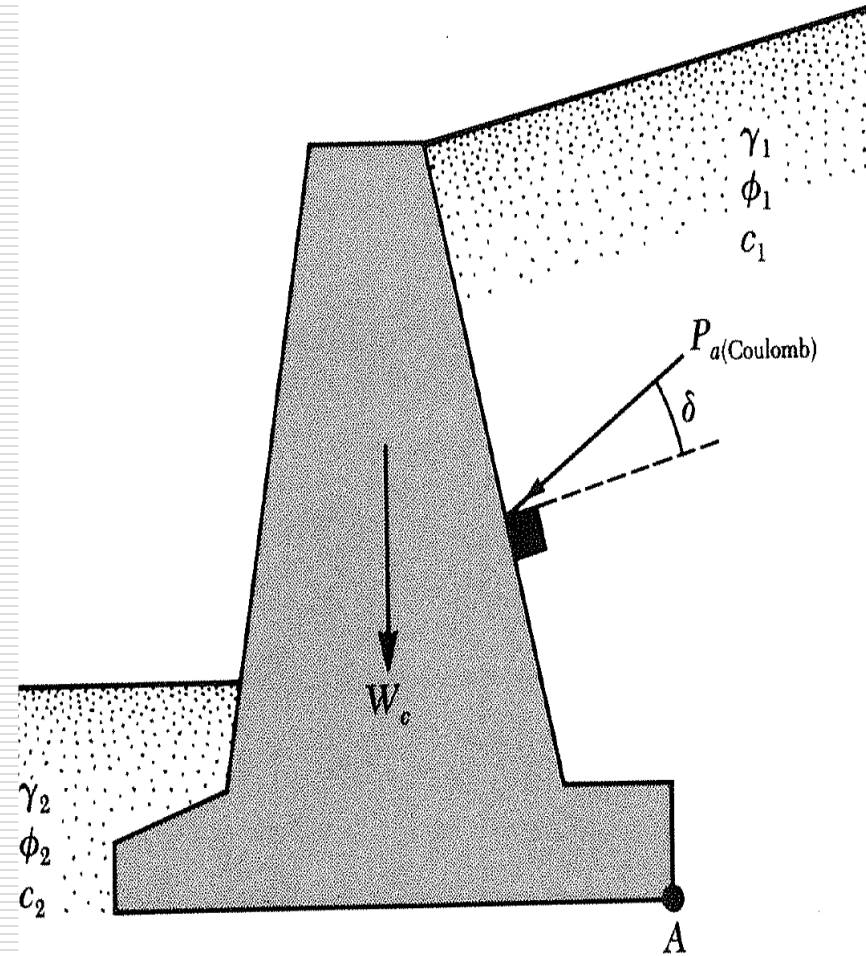
§ 5.4 Design of retaining walls

Stability checks for retaining walls

□ Assumptions to check the stability of retaining walls in case of applying the Coulomb's theory:

- The forces to be considered are the active force P_a and the RW weight W_c
- The active pressure is considered as acting on the internal face of the wall.
- The angle δ made with the normal on the wall edge represents the friction angle between the soil and the retaining surface:

$$\delta = (1/2 \dots 1/3)\Phi$$

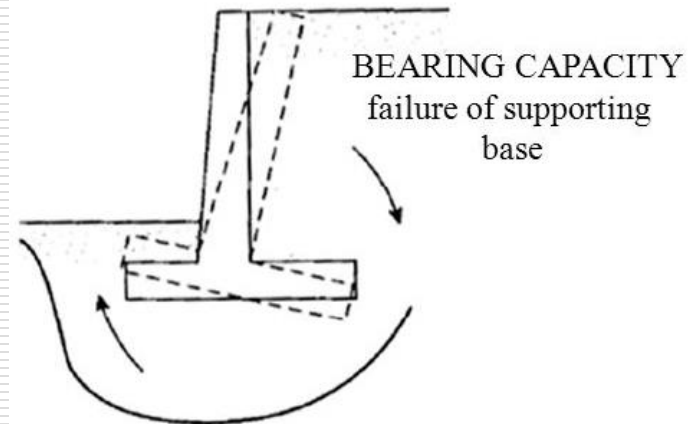
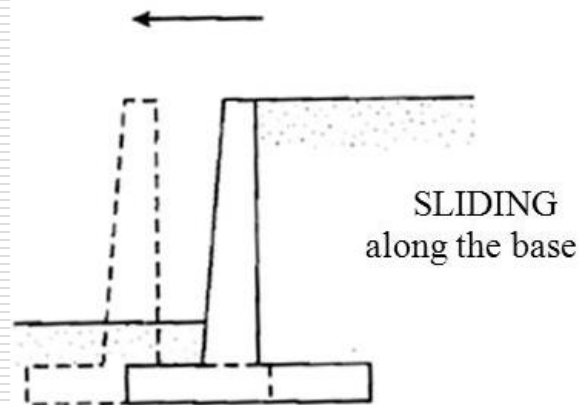


§ 5.4 Design of retaining walls

Stability checks for retaining walls

□ To check the stability of retaining walls, the following checks are necessary:

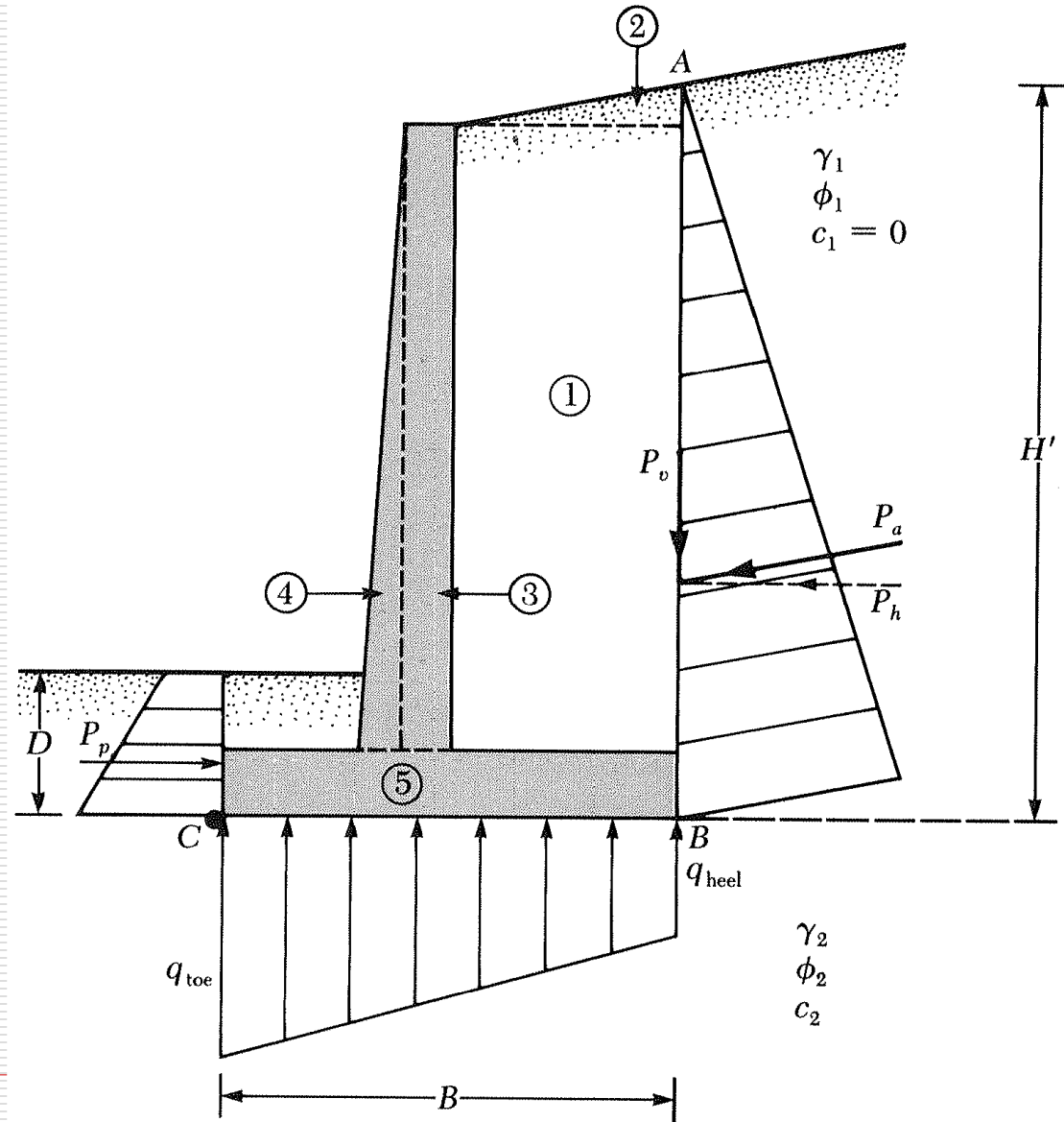
- Check for overturning about toe
- Check for sliding along its base
- Check for bearing capacity failure at the base
- Check for settlement
- Check for overall stability



§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for overturning

- The figure shows the forces acting on a cantilever (gravity) retaining wall.
- The Rankine Active Pressure P_a acts on the vertical plane AB drawn from the heel.
- On the same principles, the Rankine Passive Pressure P_p acts on the vertical plane C drawn from the toe.



$$P_p = \frac{1}{2} \cdot K_p \gamma_2 \cdot D^2 + 2c \cdot H \cdot \sqrt{K_p}$$

§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for overturning

□ The factor of safety against overturning about point C is given as:

$$FS = \frac{\Sigma M_R}{\Sigma M_O}$$

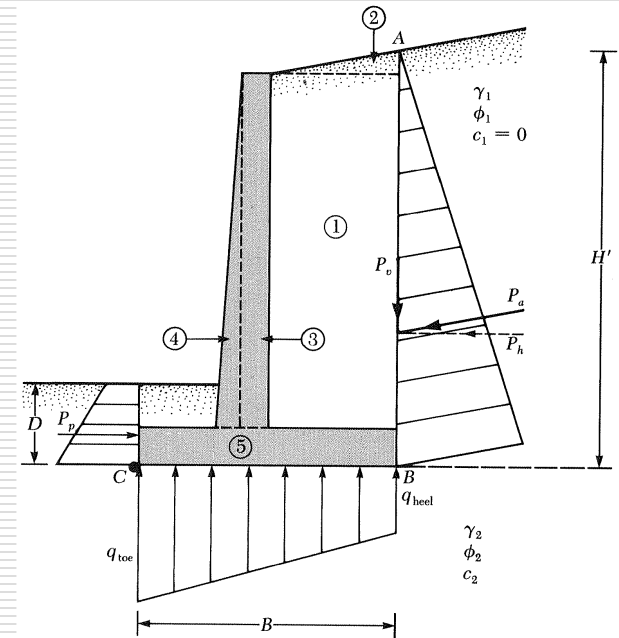
where:

ΣM_R - sum of moments of forces tending to resist overturning about point C

ΣM_O - sum of moments of forces tending to overturn about point C

$$P_h = P_a \cdot \cos \alpha \quad \Sigma M_O = P_h \cdot H/3$$

$$P_v = P_a \cdot \sin \alpha \quad \Sigma M_R = \Sigma W \cdot d + P_v \cdot B + P_p \cdot d_p$$



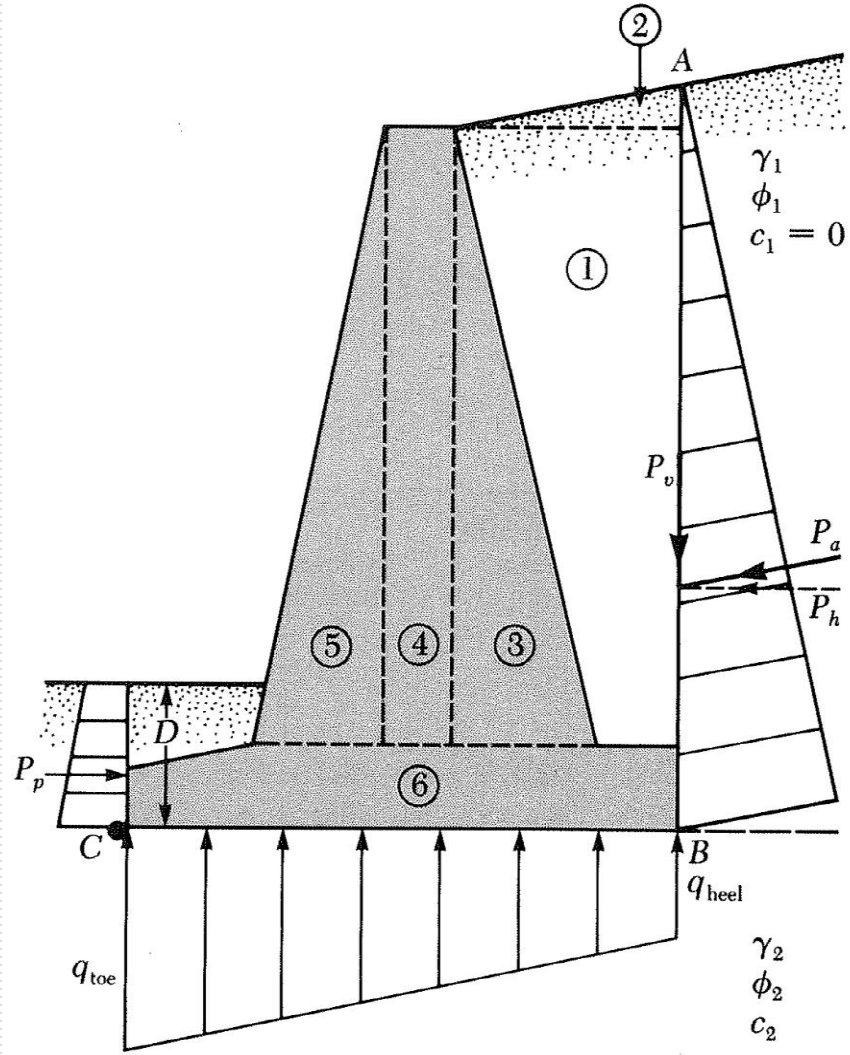
□ The design should be repeated for each action combination (see C1)

Obs: The usual minimum desirable values for the factor of safety with respect to overturning is 1.5...2.

§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for overturning

□ Check of overturning of gravity RW



§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for overturning

**Failure of RW
by
overturning:**



§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for sliding

□ The factor of safety for sliding may be expressed

as:

$$FS_{(\text{sliding})} = \frac{\Sigma F_R}{\Sigma F_d}$$

where:

ΣF_R - sum of the horizontal resisting forces

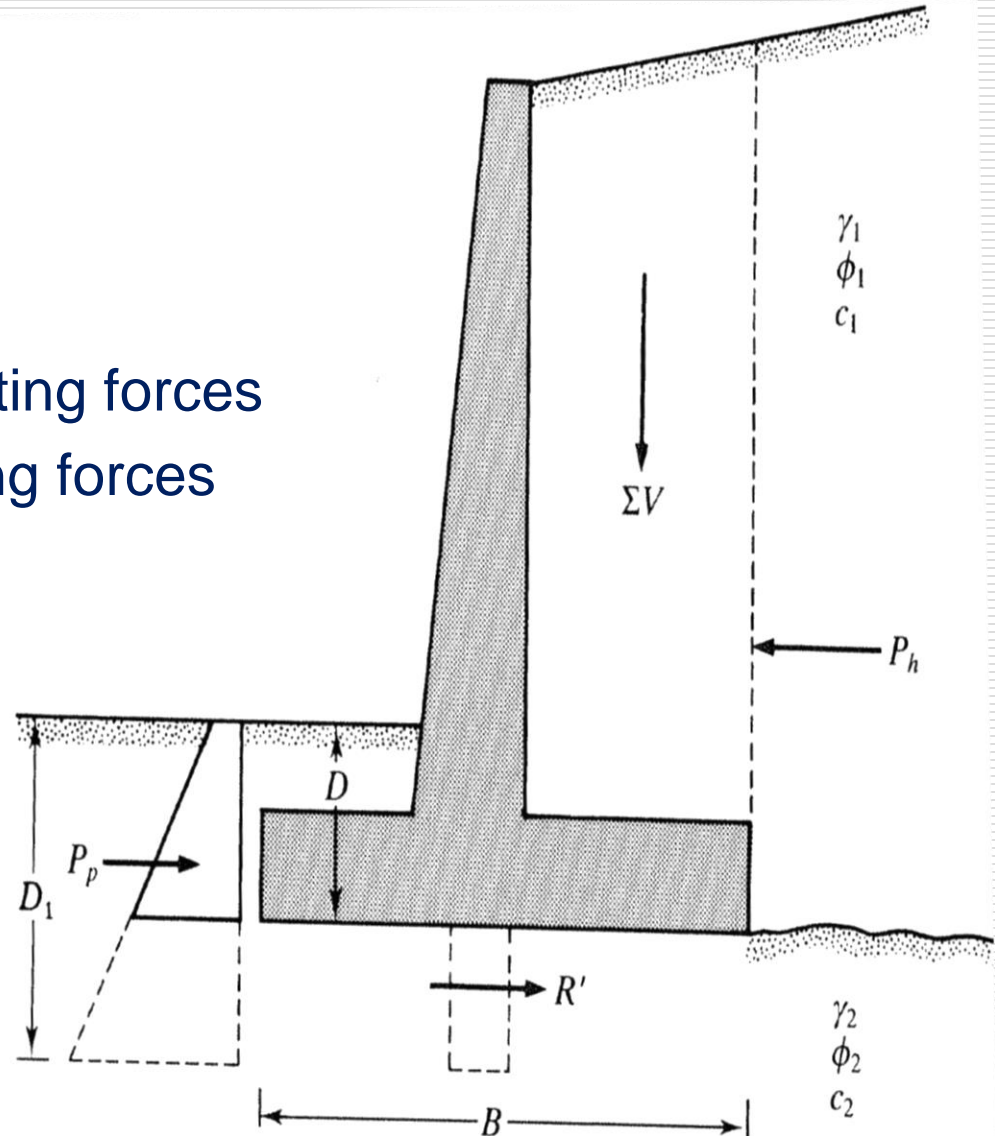
ΣF_d - sum of the horizontal driving forces

$$\Sigma F_d = P_h = P_a \cdot \cos \alpha$$

$$\Sigma F_R = R' + P_p$$

where:

R' is the maximum resisting force developed at the interface between the base slab and the bottom soil



§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for sliding

$$R' = \tau \cdot B = B \cdot (\sigma \cdot \tan \Phi_2 + c_2) = B \cdot \sigma \cdot \tan \Phi_2 + B \cdot c_2$$

but:

$$B \cdot \sigma = \Sigma V - \text{sum of the vertical forces}$$

It results:

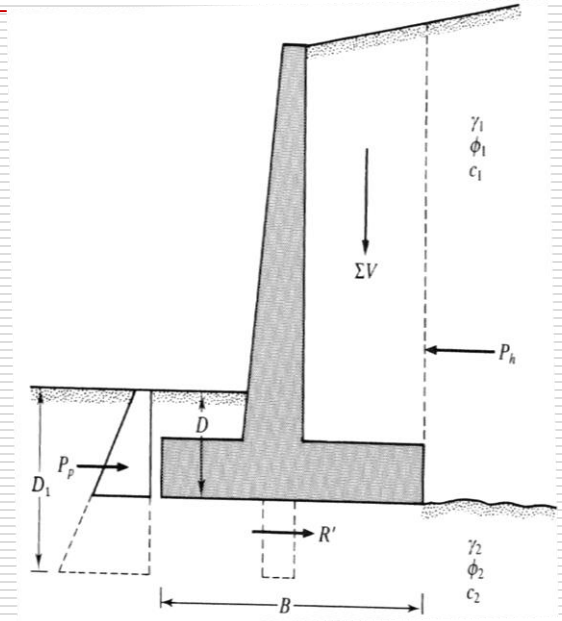
$$\Sigma F_R = (\Sigma V) \cdot \tan \Phi_2 + B \cdot c_2 + P_p$$

□ In consequence, the factor of safety for sliding may be expressed as:

$$FS_{(\text{sliding})} = \frac{(\Sigma V) \cdot \tan \Phi_2 + B \cdot c_2 + P_p}{P_a \cdot \cos \alpha}$$

Obs: The usual minimum desirable values for the factor of safety with respect to overturning is 1.0 in accordance to EN 1997-1, as the partial safety factors were included in the design combination of actions.

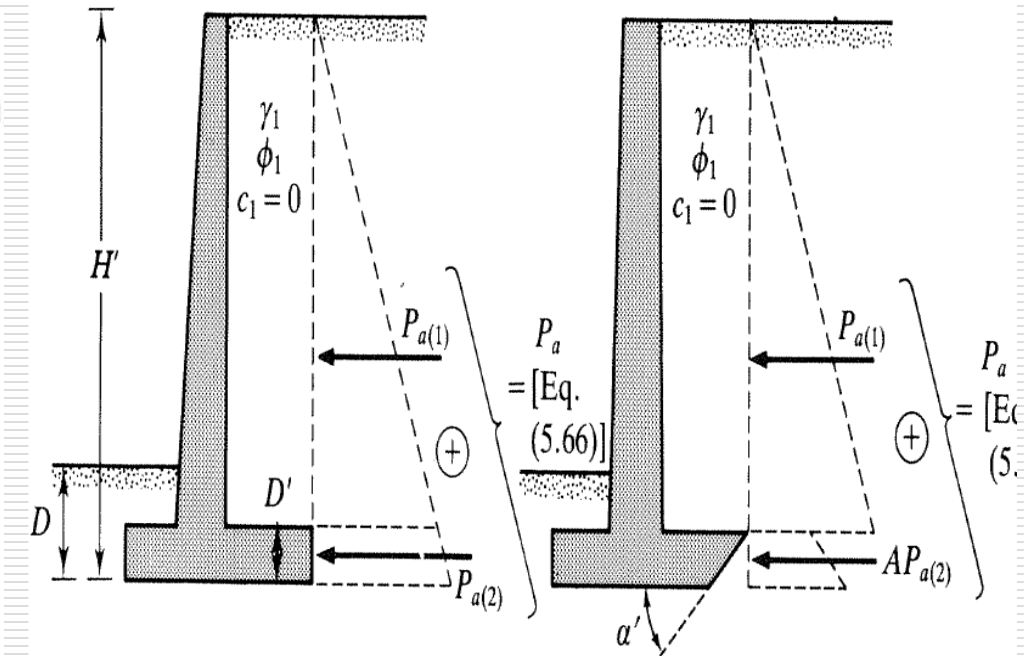
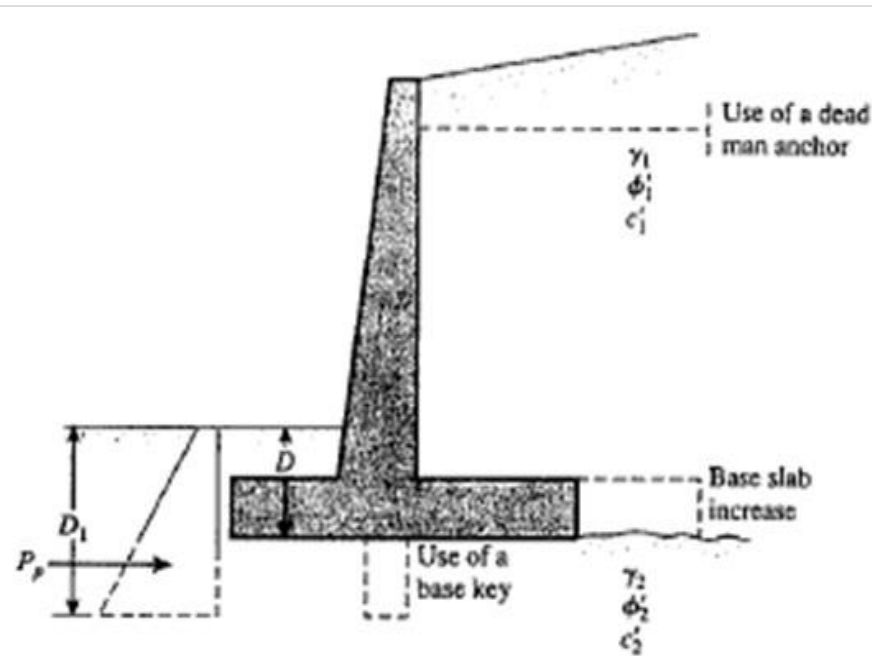
Obs 2: In many cases, the passive force P_p is ignored in calculation.



§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for sliding

□ In many cases, the checking for sliding is not fulfilled. In order to improve the stability to sliding, we can adopt one of the following:



□ Use of a base key: this will generate an additional passive pressure (resisting force) on the key height;

□ Modification of the shape of the supporting surface (reducing the active pressure on the base slab);

§ 5.4 Design of retaining walls

Stability checks for retaining walls – check for sliding

Failure of RW by sliding:

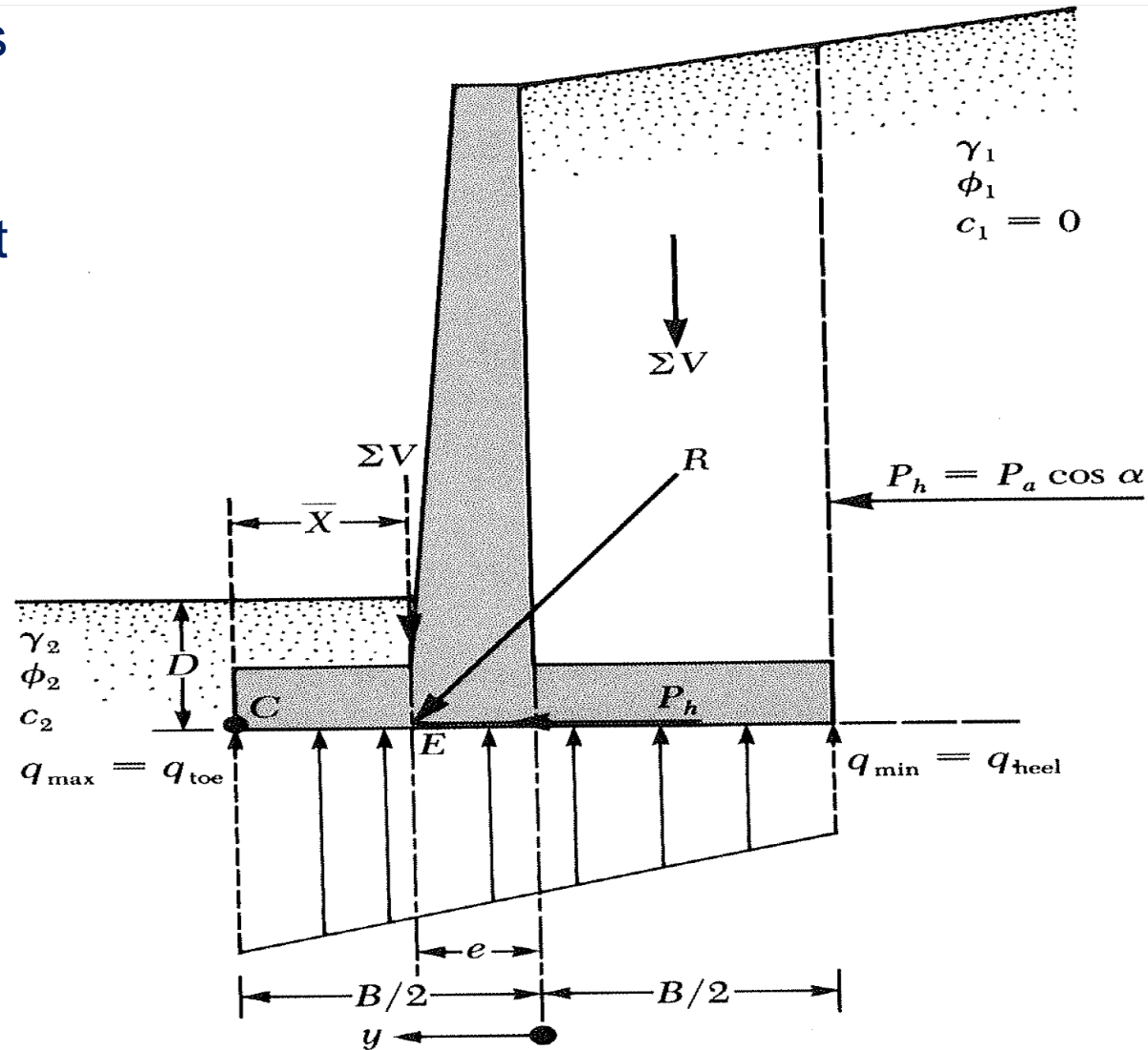


§ 5.4 Design of retaining walls

Stability checks for RW – check for bearing capacity failure

□ The vertical pressure as transmitted to the soil by the base slab of the RW should be checked against the ultimate bearing capacity of the soil.

□ q_{toe} and q_{heel} represents the maximum and the minimum pressures occurring at the limits of the toe and heel sections.



§ 5.4 Design of retaining walls

Stability checks for RW – check for bearing capacity failure

□ The values of q_{toe} and q_{heel} can be determined in the following manner:

- All the loads are reduced in a point on the basis of the foundation (e.g. center);
- There could be computed the sum of vertical forces:

$$V = \Sigma W_i + Pav$$

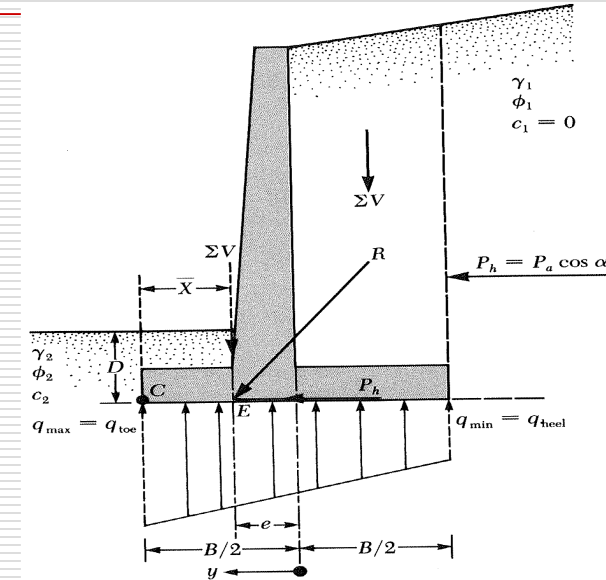
□ The net moment of the foundation is: $M_{net} = \Sigma M_R - \Sigma M_O$

□ The values of q (σ) results from: $q_{max/min} = \frac{V}{A} \pm \frac{M_{net}}{W} = \frac{V}{B \cdot 1} \pm \frac{M_{net}}{B^2 \cdot 1/6}$

□ Checking of the bearing capacity failure implies:

$$q_{max} \leq q_{conv}$$

Obs: Usually the entire base slab should be in compression.



§ 5.4 Design of retaining walls

Stability checks for RW – check for bearing capacity failure

Failure of RW by overpassing the bearing capacity:

