

Solute pollutants transport modelling in Amarastii de Sus village, Dolj County – Romania

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Abstract: The Paper present unconfined groundwater flow and solute transport modelling Amarastii de Sus village, using the PMWIN applications. Modelling purpose is to constitute extending pollution zone of aquifers in space and time generated of wastewater treatment plant. Knowledge of extending of pollution zone is necessary to settle technical measures to closing nonconformity landfill. The companion software Processing Modflow for Windows (PMWIN) offer a totally integrated simulation system for modelling groundwater flow and transport processes with MODFLOW-88, MODFLOW-96, PMPATH, MT3D, MT3DMS, MOC3D, PEST and UCODE.

Keywords: Hydraulic modelling, pollutants transport

necessary to ensure the flow and pressure regime.

In the basement room of valves: pumping station will be made of three pumps 2 + 1 R Q = 18 m³ / h, H = 57 m, P = 5.5 kW and a fire pump Q = 24 m³ / h, H m = 57, P = 7.5 kW and hydraulic installations required (4 valves separation - rubber butterfly valve, non-return valves 4, 4 valves pump selection - rubber butterfly valve, all DN 50, PN25). The plant is fully automated switchboard by AER variable speed pumps operated via frequency inverter controlled flow out of Plant and read the pressure in the distribution system of meter reading in the home.

1. INTRODUCTION

Amarastii de Sus village is located in the south east of Dolj County, in an area of lowland agricultural purposes, the county road DJ 604 at about 60 km from the city of Craiova (Figure 1).

Pipeline routes are located partly on area roadways, partly in the green area in front of the houses.

The location of the proposed pipeline route will be the same as far as possible, with all streets.

The pipes will be laid in the ground at an average depth of 1.20 m below freezing. To ensure water consumers in the city, take into account a degree of assurance of water use by $cs = 0.95$ and a maximum allowable pressure in the network of 60 m H₂O.

Homes valves to be performed (35 pcs.) Will be concrete inner diameter of 1250 mm (29 pieces) respectively the inner diameter of 1500 mm (6 pieces).

According hydrogeological study prepared by C.N. Romanian waters for water supply source is indicated shallow aquifer complex, stationed in the range 25-50 m. The aquifer can provide a flow rate of 5 l / s through execution and completion of drilling correct.

By study recommends two wells with depth of 50 m, completed with a single column De.250x11.9 mm, confined aquifer layers that will capture the 25-50 m range, about 3-4 layers. Surface active filters will be over 8%.

Exploration drilling operation will team with one submersible pump the following characteristics Q = 5 cm / h, H = 45 mWS, P = 2.2 kW.

Over drilling runs homes for protection of concrete, housing helmet shaft drilled and hydraulic installations

2. MATERIAL AND METHODS

The applications of MODFLOW, a modular three-dimensional finite-difference groundwater model of the U. S. Geological Survey, to the description and prediction of the behavior of groundwater systems have increased significantly over the last few years.

Models or programs can be stand-alone codes or can be integrated with MODFLOW. A standalone model or program communicates with MODFLOW through data files. The advective transport model PMPATH (Chiang and Kinzelbach, 1994, 1998), the solute transport model MT3D (Zheng, 1990), MT3DMS (Zheng and Wang, 1998) and the parameter estimation programs PEST (Doherty et al., 1994) and UCODE (Poeter and Hill, 1998) use this approach.

The solute transport model MOC3D (Konikow et al., 1996) and the inverse model MODFLOWP (Hill, 1992) are integrated with MODFLOW. Both codes use MODFLOW as a function for calculating flow fields.

The companion software Processing Modflow for Windows (PMWIN) offer a totally integrated simulation system for modeling groundwater flow and transport processes with MODFLOW-88, MODFLOW-96, PMPATH, MT3D, MT3DMS, MOC3D, PEST and UCODE.

PMWIN comes with a professional graphical user-interface, the supported models and programs

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expands and contracts in response to changes in reservoir stage. The Stream flow-Routing package (Prudic, 1988) was designed to account for the amount of flow in streams and to simulate the interaction between surface streams and groundwater.

In the block-centered finite difference method, an aquifer system is replaced by a discretized domain consisting of an array of nodes and associated finite difference blocks (cells).

The nodal grid forms the framework of the numerical model. Hydro stratigraphic units can be represented by one or more model layers. The thicknesses of each model cell and the width of each column and row can be specified. The locations of cells are described in terms of columns, rows, and layers.

MODFLOW requires initial hydraulic heads at the beginning of a flow simulation. Initial hydraulic heads at fixed-head cells will be kept constant during the flow simulation. An IBOUND array is required by the flow model MODFLOW.

The IBOUND array contains a code for each model cell. A positive value in the IBOUND array defines an active cell (the hydraulic head is computed), a negative value defines a fixed-head cell (the hydraulic head is kept fixed at a given value) and the value 0 defines an inactive cell (no flow takes place within the cell).

3. RESULTS AND DISCUSSIONS



Figure 2. Model and Discretization

As boundary conditions there are accepted:
 - concentration $C=100\%$ on the treatment plant as a permanent pollution source
 - concentration $C=0$ in the field outside of landfill area.

The dispersivity has the following characteristics:
 $\alpha L=50$; $\alpha T/\alpha L=0.1$.

Permanent source assumption is based to fact treatment plant is in function of more decades and

The tasks are:

- to calculate and show head contours,
- model calibration,
- to calculate and show time variant concentration of solute pollutants transport

The aquifer extends several square kilometers (2,1 x 1,6 km), situated in space of irrigation channel in vest and east. In map presented in Figure 2 is marked model limits.

The top and bottom elevations of the aquifer are variably 120 m and 117 m, respectively.

The average horizontal hydraulic conductivity is 0.0001 m/s; the effective porosity is 0.25.

The water stage in the irrigation channel is 118 m to 116 m above the bottom elevation, which is the reference level for the simulation.

The aquifer is simulated using a grid of one layer, 100 columns and 100 rows. A grid spacing is irregular (21 m is used for column and 15,6 m is used for row) (Figure 2). The layer type is unconfined. The irrigation channel are modeled as fixed head in the channel. One boundary is fixed hydraulic head boundaries.

Figure 3 shows the head contours.

To modeling polluted transport the pollution source is the wastewater treatment plant considered as a permanent pollution.

determinant pollution infiltration process in conjugate precipitation effects persist in many years.

Results are presented in figures under polluted extending areas with different concentrations (isoconcentrations) for 10 years (Figure 4).

The results of simulation permit exactly quantify evolution of concentrations in time, for all points of polluted zones. This think is important to find a technical method for limitation, reduce or eliminate in time pollution.

A special remark that is not necessary to specify pollutant nature, because the modeling was making in general for all kind of pollutants. The concentration values are expressed in percentage from the total quantity of pollutant.

Concentration is a general parameter and maybe serve base for calculation an absolute concentration

(for example mg/l) for all dissolved pollutants in water. From the concentrations diagrams the Local Authorities has possibility to know the directions and evolution in time of the pollution from treatment plant.

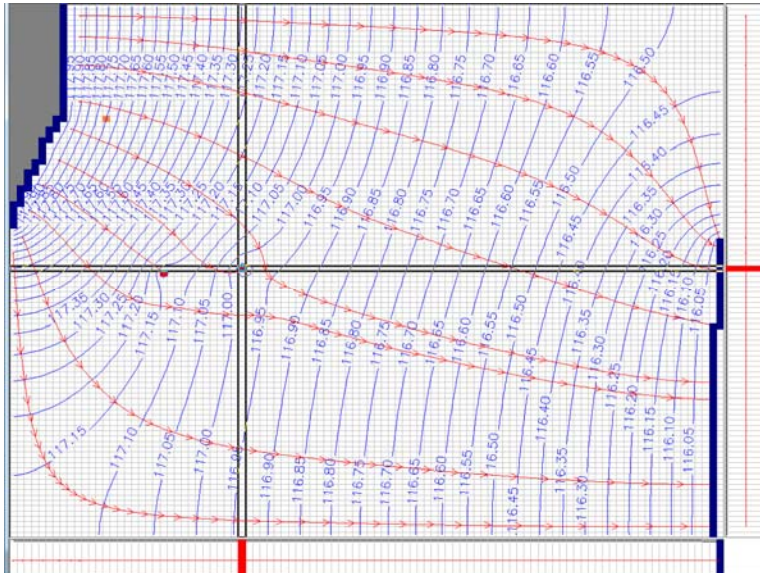


Figure 3. Head contours

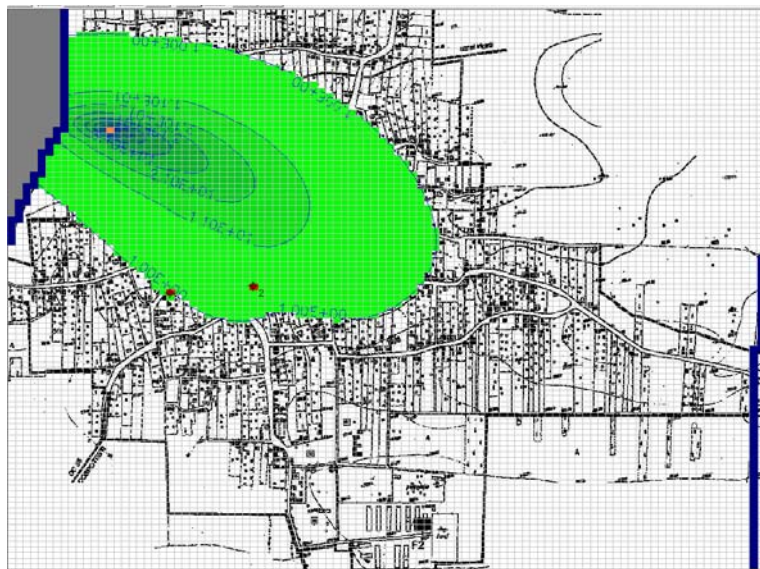


Figure 4. Distribution of the pollutant

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